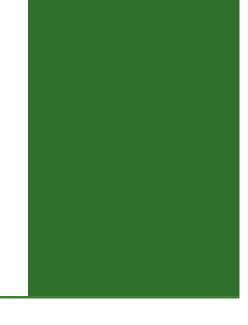
Prepared By:





# **Town of Minto**

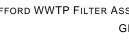
# **Clifford WWTP Filter Assessment Report**

**GMBP File: 323072** 

March 2024



March 2024





# **TABLE OF CONTENTS**

1.	INTRODUCTION	1
2.	BACKGROUND	1
3.	DESCRIPTION OF EXISTING SYSTEM	2
4.	ANALYSIS OF PLANT OPERATING DATA	6
4.1	Sewage Flows	6
4.2		
4.3	Effluent Quality Analysis	10
5.	RESULTS OF SYSTEM ASSESSMENT	14
5.1	Facultative Lagoons	14
5.2	( -)	
5.3	Filter Media Analysis	17
6.	CONCLUSIONS AND RECOMMENDATIONS	22
6.1	System Capacity	22
6.2		
6.3	Facultative Lagoons	22
6.4	Intermittent Sand Filters	22
7.	PRELIMINARY COST ESTIMATES FOR RECOMMENDATIONS	23

# **APPENDICES**

APPENDIX A: MECP ENVIRONMENTAL COMPLIANCE APPROVAL

APPENDIX B: LAB RESULTS - WASTEWATER SAMPLING

APPENDIX C: LAB RESULTS - FILTER MEDIA SAMPLING

APPENDIX D: COST ESTIMATES - DETAILED BREAKDOWN

APPENDIX E: SELECTED DESIGN DRAWINGS

GMBP File: 323072 March 2024



# **CLIFFORD WASTEWATER TREATMENT PLANT**

#### FILTER ASSESSMENT REPORT

#### **MARCH 2024**

**GMBP FILE: 323072** 

#### 1. INTRODUCTION

GM BluePlan Engineering Limited (GMBP) was retained by the Municipality of the Town of Minto (Minto) to provide professional engineering services for assessment of the existing sand filters at Clifford Wastewater Treatment Plant (WWTP). The scope of work is described in our letter proposal to Minto dated September 22, 2023. This assignment is initiated by the Town due to operating experience which is demonstrating that the sand filters are not draining properly and plant performance is declining. There are currently no Orders or Directives from the Ministry of Environment, Conservation and Parks (MECP) regarding the sewage works.

GMBP technical staff conducted a detailed site investigation with operations staff on November 7, 2023. As part of the field work, samples were collected from lagoon effluent and filter effluent. Filter media samples were also collected from several locations in one filter cell and analyzed for geotechnical properties relevant to filter performance.

#### 2. BACKGROUND

The Village of Clifford is located in northern Wellington County and has a population of approximately 760 people. The Village is serviced with municipal water and sanitary sewage services. The Clifford sewage works generally consists of a sanitary sewage collection system, raw sewage pumping station, control building for aeration blowers, chemical feed equipment and standby power, aerated lagoon cell, 3 non-aerated facultative lagoons, lagoon effluent pumping station, 4 cell intermittent sand filter, 2 filtered effluent storage lagoons, and final effluent outfall pipe to a local Municipal Drain. The plant operates on a seasonal discharge basis, with filtered effluent produced from Spring to Fall that is stored in the filtered effluent storage lagoons for discharge to the receiver from Fall to Spring, on an annual basis. The allowable final effluent discharge period is April 1 to November 30 each year.

The Clifford WWTP is captured by the Ministry of Environment, Conservation and Parks (MECP) with Environmental Compliance Approval (ECA) No. 8901-A8YJ9C dated June 7, 2016. The rated annual average flow capacity of the plant is 500 cubic metres per day (m³/d) with a peak flow capacity of 2,151 m³/d. A copy of the ECA is contained in Appendix A.

Minto provided GMBP with plant operating data for wastewater analysis for the period from January 1, 2020 to December 31, 2023. This information was analyzed as part of this assignment.

A map showing the general location of the Village of Clifford and the associated WWTP are shown below in Figure 1.

GMBP File: 323072 March 2024



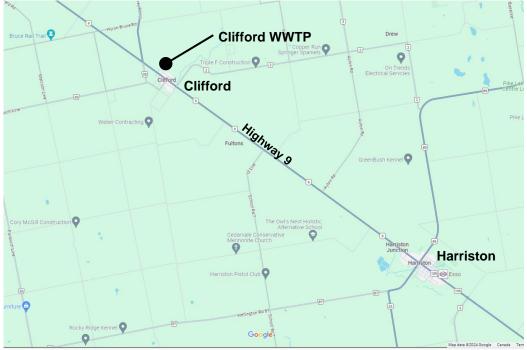


Figure 1. Site Location Map of Clifford WWTP (Google Maps, 2023)

### 3. DESCRIPTION OF EXISTING SYSTEM

Main components of the sewage works as described in the ECA are outlined below.

- 1. Sanitary Sewer Collection System
  - a. List of the streets in Clifford with sanitary sewers
- 2. Influent Pumping Station
  - a. One (1) 3.0m diameter wet well;
  - b. Duplex submersible sewage pumps, each rated at 29 L/s at 15 m TDH;
  - c. Basket screen with hoisting equipment.
- 3. Control Building, contains
  - a. 2 blowers rated at 99 L/s at 41 kPa for the aerated lagoon cell;
  - b. Chemical feed system with duplex metering pumps and 27,000 L chemical storage tank;
  - c. 60kW standby diesel generator with 908 litre fuel tank.
- 4. Aerated Facultative Lagoon
  - a. One (1) cell with surface area of approx. 1,400 m2 and total storage volume of 2,000 m3;
  - b. Equipped with coarse air bubble diffusion, inlet and outlet chambers;
  - c. Discharging to Facultative Lagoon 1.
- 5. Facultative Lagoons (3)
  - a. Lagoon Cell 1 with a surface area of approx. 22,300 m2 and storage volume of 35,000 m3;
  - b. Lagoon Cell 2 with a surface area of approx. 24,500 m2 and storage volume of 40,000 m3;
  - c. Lagoon Cell 3 with a surface area of approx. 33,900 m2 and storage volume of 75,000 m3;
  - d. Total facultative lagoon storage volume of 150,000 m3
- 6. Lagoon Effluent Pump Station
  - a. Underground concrete pumping chamber installed at edge of Facultative Lagoon 3;







b. Equipped with four (4) submersible pumps, each rated at 30 L/s at 5.2 m TDH discharging to a 200mm forcemains feeding each sand filter.

### 7. Intermittent Sand Filter (ISF)

- a. Consisting of four (4) ISF cells, each with a surface area of 460 m<sup>2</sup>;
- b. Each filter contains a layer of sand filter media 760mm thick, filter influent piping and valves, filter distribution piping, filter underdrain piping, effluent sewer and manholes;
- c. Discharges filtered effluent via a Filtered Effluent Distribution Chamber to Filtered Effluent Storage Lagoon 1.

### 8. Filtered Effluent Storage Lagoons

a. Two (2) filtered effluent storage lagoons, each with surface area of approx. 42,900 m2, liquid depth of 2.4m for a total storage capacity of 187,600 m3.

#### 9. Effluent Outfall

a. One (1) 200mm diameter outfall sewer discharging to an open ditch on the property, ultimately flowing to Raccoon Creek via Minto Municipal Drain 93.





GM BluePlan Engineering (formerly Gamsby and Mannerow Ltd.) was the original design engineer for the Clifford WWTP. The plant was constructed in 1995. Selected design drawings are included in Appendix E that show the overall treatment process and details of the sand filters.

The following table summarizes monitoring requirements and effluent quality criteria stipulated in the ECA.

Table 1. Summary of ECA Parameters and Monitoring Requirements

Parameter	Monitoring Raw Sewage	Monitoring Effluent (1.)	Objectives (mg/L)	Limits (mg/L)
5-day Biochemical Oxygen Demand (BOD5)	weekly	n/a	n/a	n/a
5-day Carbonaceous Biochemical Oxygen Demand (cBOD5)	n/a	weekly	7.5	12.0
Total Suspended Solids (TSS)	weekly	weekly	7.5	12.0
Total Phosphorous (TP)	weekly	weekly	0.7	0.7
Total Kjeldahl Nitrogen (TKN)	weekly	n/a	n/a	n/a
Total Ammonia Nitrogen (TAN)	n/a	weekly	2.0 (Fall, Spring) 4.0 (Winter)	3.0 (Fall, Spring) 6.0 (Winter)
Un-ionized Ammonia Nitrogen NH3 (calculated)	n/a	weekly	0.02	n/a
E. Coli	n/a	weekly	100 CFU/100mL	n/a
Dissolved Oxygen (DO)	n/a	weekly	=> 5.0	n/a
рН	n/a	weekly	6.0 to 9.5	n/a
Temperature	n/a	weekly	n/a	n/a

### Notes

- 1. Effluent quality monitoring is required only during discharge period.
- 2. n/a : not applicable.

The system operates on a seasonal discharge basis. Final effluent can only be discharged from November 1 to April 30 annually and only during a non-freezing period. Effluent monitoring is conducted weekly during the approved discharge period only.



An aerial view of the Clifford WWTP with the main components of the system are shown below in Figure 2.

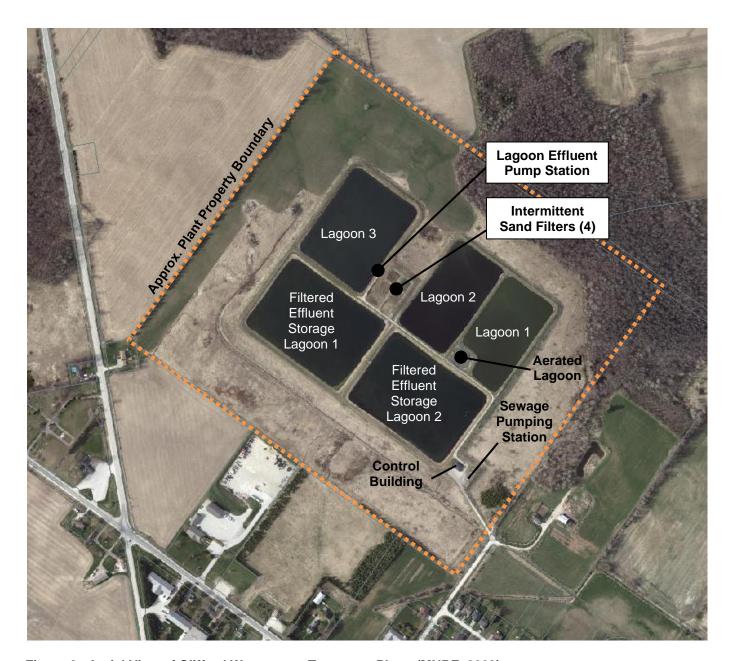


Figure 2. Aerial View of Clifford Wastewater Treatment Plant (MNRF, 2023)



#### 4. ANALYSIS OF PLANT OPERATING DATA

# 4.1 Sewage Flows

The final effluent discharge period is defined as November 1 to April 30 annually, due to the available assimilative capacity of the receiving stream. Flow rates in the receiving stream are typically at their seasonal minimal level during summer months. However, the sand filters can only be operated during non-freezing periods. Consequently, original design of the treatment train included 2 filtered effluent storage lagoons with a large storage volume to hold filtered effluent annually from May 1 to October 31.

The following table summarizes allowable discharge volumes of final effluent to the receiver for the Clifford WWTP along with measured discharge volumes for the period from January 1, 2020 to September 30, 2023.

Table 2. Summary of Reported Sewage Flows

Month	Allowable Discharge (m3/day)	Allowable Discharge (m3/month)	Monthly Averages (m3/month)	Monthly Maximums (m3/month)
November	470	14,100	6,086	8,300
December	734	22,754	13,743	17,182
January	1,028	31,868	22,933	26,889
February	1,075	30,100	21,348	25,080
March	1,970	61,070	22,003	27,453
April	1,345	40,350	10,562	22,446
May to October	0	0	0	0
ANNUAL TOTALS		200,242 m <sup>3</sup>	96,675 m³	

The 95th percentile and absolute maximum recorded daily discharge volumes from the data set were 899 m3/day and 998 m3/day, respectively. These values are well below the maximum allowable daily value of 2,151 m3/day indicated in the ECA. Monthly flow volume indicated in Table 2 above are all below allowable limits. Review of effluent discharge data indicates that the plant has been consistently operating well below maximum allowable discharge rates.



Figure 3 below illustrates sewage flow data for the period from January 1, 2020 to September 30, 2023.

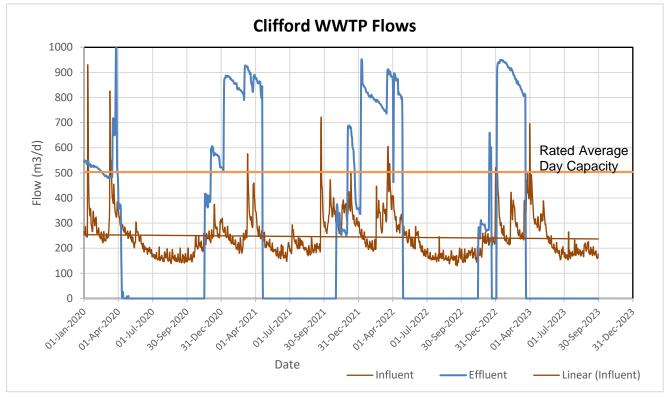


Figure 3. Sewage Flow Profile

The linear trend line indicated in Figure 3 above is nearly flat, indicating no apparent increasing or decreasing trend in raw sewage flows arriving at the plant since January 2020.

### 4.2 Raw Sewage Quality Analysis

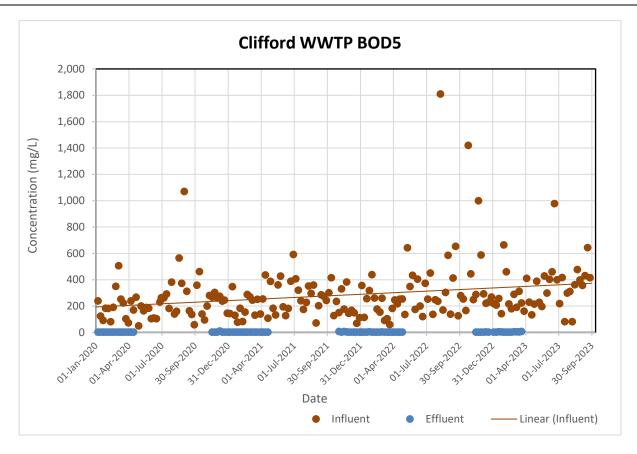
The intent of reviewing raw sewage strength is to determine if there is a trend or change that may be contributing to the observed decline in final effluent quality. Analysis of plant operating data indicates that the raw sewage concentration for  $BOD_5$  is increasing gradually, while raw sewage concentrations for TSS, TP, and TAN are increasing moderately. See graphs below in Figure 4. It is noted that final effluent concentrations for CBOD5, TSS, TAN, and TP have been increasing in 2022 and 2023.

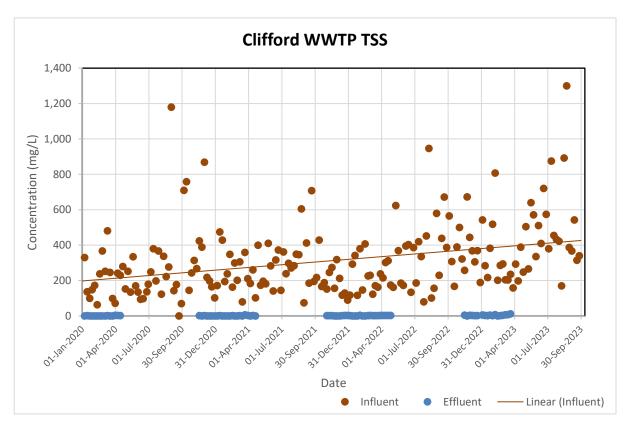
Reasons for an observed trend in increasing raw sewage strength may include implementation of water conservation measures such as low-flow plumbing fixtures, and a reduction in extraneous flows (inflow and infiltration) into the sanitary sewer collection system. There may be a new industrial or commercial customer in Clifford that is discharging high-strength waste to the system, although that may manifest as a step increase rather than a gradual long-term trend.

The following 4 graphs illustrate raw sewage quality monitoring results for the period from January 1, 2020 to September 30, 2023. The solid line is a linear regression line to indicate the trend in raw sewage strength for each parameter.



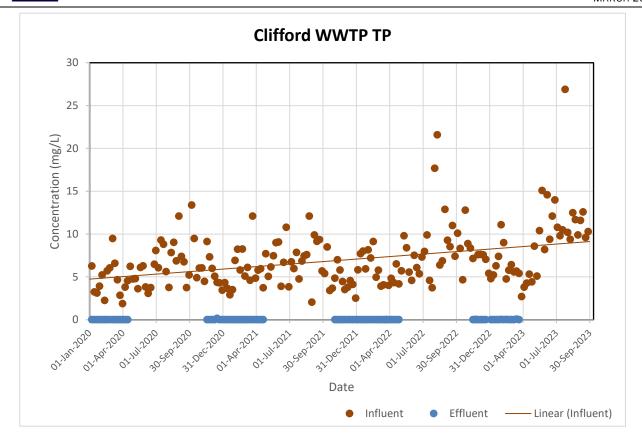
March 2024











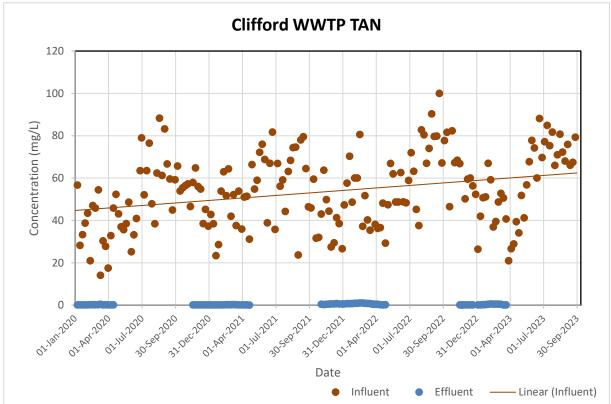
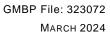


Figure 4: Raw Sewage Quality Graphs





The following table summarizes lab results for key wastewater parameters in raw sewage along with design guideline values published by MECP Design Guidelines (2008) and by Metcalf and Eddy Wastewater Engineering (5th Ed.) for untreated domestic wastewater.

Table 3. Comparison of Raw Sewage Characteristics to Design Values (mg/L)

Parameter	Clifford (Jan. 1, 2020 to Sept. 30, 2023)	MECP Design Guidelines	M&E Wastewater Engineering (1.)	M&E Wastewater Engineering (2.)
BOD5	284	155 to 286	190	350
TSS	314	155 to 330	210	400
TKN	66	26 to 75	40	70
TAN	54	4 to 13	25	45
Organic N	12	22 to 62	15	25
TP	7	6 to 12	7	12

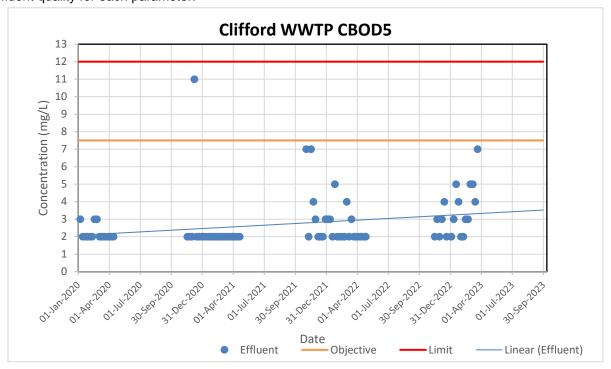
#### Notes

- Medium Strength Wastewater
- 2. High Strength Wastewater

Based on a comparison of average values for Clifford to published design guideline values, raw sewage at Clifford could be considered as medium to high strength wastewater in terms of BOD5, TSS, and Total Ammonia Nitrogen, and medium strength sewage in terms of organic nitrogen and Total Phosphorous.

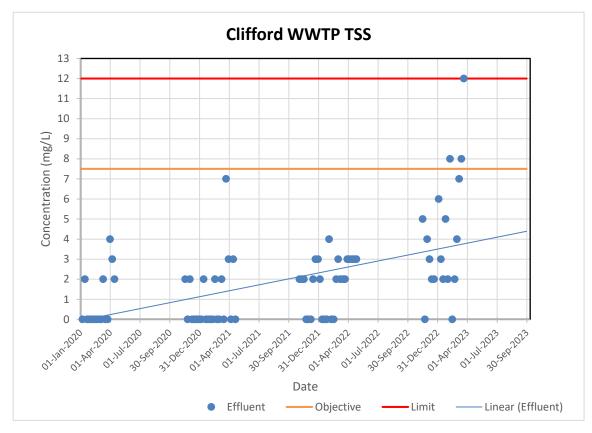
### 4.3 Effluent Quality Analysis

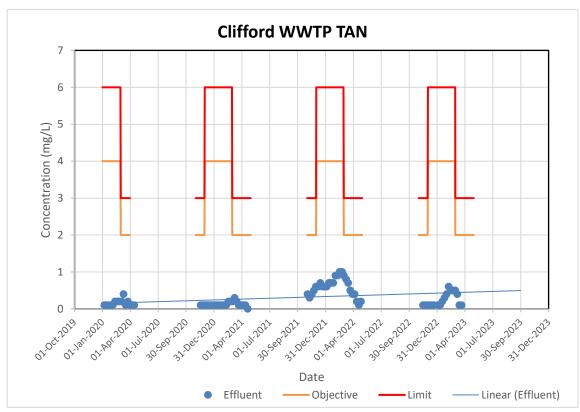
The following 7 graphs illustrate final effluent quality monitoring results for the period from January 1, 2020 to September 30, 2023 for key wastewater parameters. The solid line is a linear regression line to indicate the trend in effluent quality for each parameter.





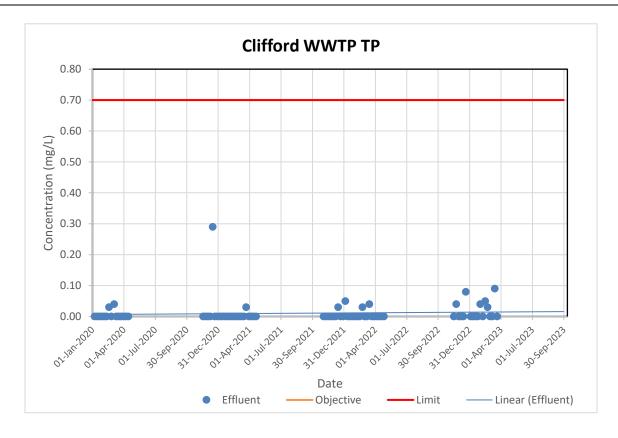


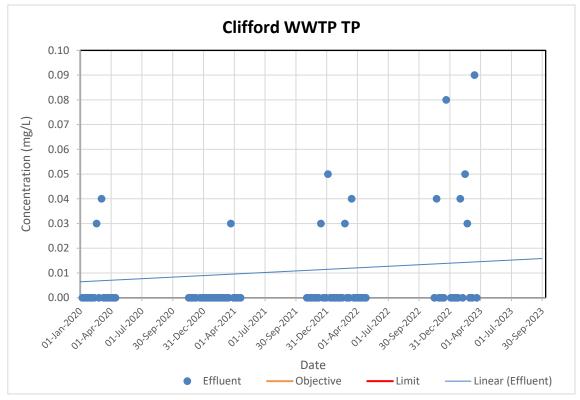






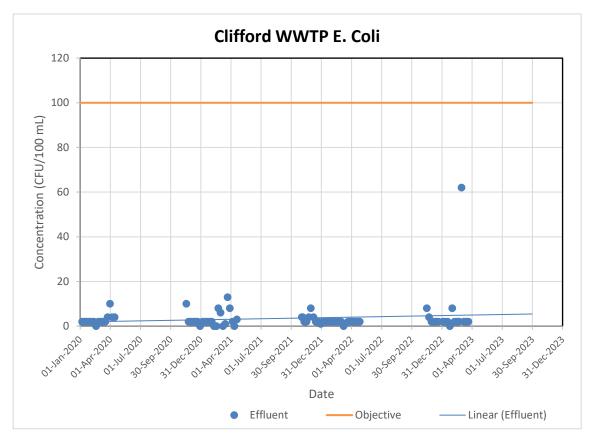
March 2024





Second TP graph with a small vertical scale to show detail.





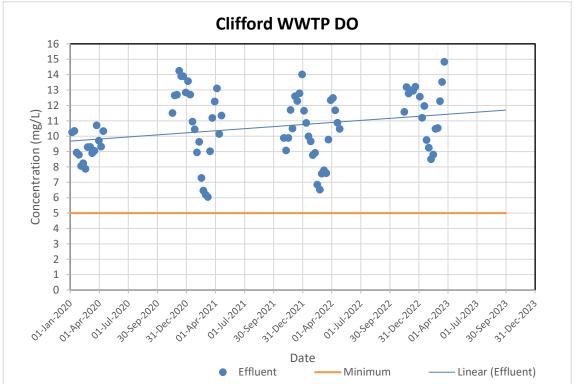


Figure 5: Final Effluent Quality Graphs





Key observations from review of effluent quality data for the period noted above indicate that effluent quality has been gradually deteriorating for the following parameters:  $BOD_5$ , TSS, TAN, and TP. Effluent quality has remained relatively consistent for E. coli. In addition, concentrations of TSS occasionally exceed effluent quality objectives and one occurrence where the limit was exceeded. The concentration of effluent  $BOD_5$  exceeded the effluent objective in late 2020, but that appears to be an outlier and not part of the recent trends. Although effluent quality for the other parameters has been declining, test results are still low with no exceedances of limits or objectives during the above noted time period, with the exception of TSS in March 2021 which exceeded the effluent objective of 7.5 mg/L. TP concentrations in final effluent have been increasing in 2022 and 2023, however actual concentrations remain well below the effluent quality criteria for both the objective and limit.

Figures 4 and 5 above present a series of graphs illustrating lab results for wastewater sampling for raw sewage and final effluent, respectively. Each graph also includes a linear trend line to determine if there is a decreasing, steady, or increasing trend in wastewater quality characteristics. Results of that analysis are summarized in the following table.

**Table 4. Observed Trends in Wastewater Quality** 

Parameter	Raw Sewage	Final Effluent
cBOD₅	Increasing	Increasing
TSS	Increasing	Increasing
TAN	Increasing	Increasing
TP	Increasing	Increasing
E. Coli	n/a	No change

#### Notes

Declining effluent quality for BOD<sub>5</sub>, TSS, TAN, and TP could be attributed in part to a corresponding increase in raw sewage strength for those parameters.

#### 5. RESULTS OF SYSTEM ASSESSMENT

During our site visit on November 7, 2023, samples of wastewater and filter media were collected for analysis at environmental labs. See Appendix B for lab results from analysis of wastewater samples and Appendix C for lab results from analysis of sand filter media. A summary of the findings is described in the following sections.

### 5.1 Facultative Lagoons

This section presents a cursory analysis of lagoon performance based on available data. The intent is to determine if lagoon performance is contributing to declining final effluent quality.

The system consists of an aerated lagoon cell followed by 3 much larger facultative lagoons, all operating in series. Solids settle to the bottom of each lagoon, while partially treated effluent is discharged to the next lagoon in series. Lagoon Cell 3 discharges by gravity to the Lagoon Effluent Pump Station that doses the filters.

The following table summarizes basis of design values from MECP Design Guidelines along with the actual lab results and corresponding calculated values for organic loading rates. Average raw BOD₅ for the data period indicated in the table below was 284 mg/L.

<sup>1.</sup> Data used in the above table is from lab results provided for the period from January 1, 2020 to September 30, 2023.

GMBP File: 323072 March 2024



Table 5. Basis of Design for Facultative Lagoons

	Organic Loa	Lagoon Effluent			
Source of Information	(kg/h	BOD <sub>5</sub>	TSS	TP	
	Lagoon 1 (1.) Lagoon 1 (2.)		(mg/L)	(mg/L)	(mg/L)
Clifford (Jan. 1, 2020 to Sept. 30, 2023)	31	64	n/a	n/a	n/a
Clifford (Nov. 7, 2023)	n/a	n/a	6	10	0.20
MECP Design Guidelines	22	22	25	30	1.00

#### Notes:

- 1. Based on average influent flow rate of 245 m3/d.
- 2. Based on plant rated capacity of 500 m3/d.

Calculated organic loading rates to Lagoon Cell 1 in the above table are based on an average raw sewage BOD<sub>5</sub> concentration of 284 mg/L. The above calculations appear to indicate that Lagoon Cell 1 is undersized. However, the calculations do not take into account the aerated lagoon ahead of Lagoon Cell 1. The aerated lagoon cell is aerated continuously year-round, and is expected to reduced BOD<sub>5</sub> by 50 to 70%. Based on this, it appears that the lagoons are sized appropriately for organic loading. Based on a single grab sample collected on November 7, 2023, effluent quality from Lagoon 3 is very good, with concentrations of key wastewater parameters well below MECP Guidelines.

Operations staff reported that the depth of the sludge blanket in Lagoon Cells 1 and 2 were measured in 2018 and the depth of sludge was low, with some mounding at the lagoon inlets, as expected.

# 5.2 Intermittent Sand Filters (IFS)

The filters at the plant are classified as intermittent slow sand filters (ISSF) and are a common treatment step used to provide tertiary treatment to lagoon effluent for direct surface discharge. Operating experience with sewage systems consisting of lagoons followed by intermittent sand filters in Southern Ontario indicates that filter performance is generally good and can remain consistent with routine maintenance, but can gradually decline over time and require some form of rehabilitation or replacement. Intermittent sand filters by design cannot be routinely backwashed like manufactured rapid sand filtration systems. Consequently, ISSF's eventually require media replenishment or replacement at some point in their design life.

The sewage works includes 4 filter cells with a total surface are of 1,840m<sup>2</sup>. The approx. dimensions of each filter cell are 20m east-west by 23m north-south and each filter cell has 5 parallel runs of above-ground perforated distribution pipe. Secondary effluent from Lagoon 3 is gravity fed into a lagoon effluent pumping station. The pump station has 4 submersible effluent pumps, each dedicated to intermittently dosing one filter cell on a timed basis. Filtered effluent is collected by a network of filter underdrain piping and is directed to a 200mm diameter gravity outfall pipe, which discharges to an open swale leading to a municipal drain.

The sand filters were installed in 1995 and there have been no media replenishment or replacement since that time. System operators regularly remove vegetation that grows of the surface of the media. Based on our test pits, it appears that the total depth of sand media is approx. 475mm to 550mm, compared to the original installed depth of 760mm. Vegetation was removed and the surface of the sand media raked in 2012 and 2015. In 2023, the top 300mm of filter sand were removed, but not replenished with new sand. This results in a sand filter bed depth that is less than the original design thickness of 760 mm.



MARCH 2024

The sewage works operates based on seasonal discharge. The filters are typically operated annually from May 1 to October 31, depending on the weather. Filtered effluent is not immediately discharged to the receiver. It is stored on-site in 2 filtered effluent storage lagoons for Fall to Spring final effluent discharge. The filters are dosed by the lagoon effluent pumping station on an intermittent timed basis. Typical operation involves each filter going through 8 one-hour cycles per day, with one cycle consisting of a 15-minute dosage period followed by a 45-minute rest period, resulting in a cumulative daily dosage time of 2 hours for each filter.

A comparison of Lagoon 3 wastewater quality versus final effluent quality is summarized in the following table. This table provides a general indication of performance of the filters to improve and polish lagoon effluent.

Table 6. Comparison of Lagoon Effluent and Filter Effluent Quality

	Effluent Conce	entration (mg/L)	% Reduction	ECA Objective
Parameter	Lagoon 3	Filter	Through Filters	(mg/L)
BOD₅	6	5	17	n/a
cBOD₅	5	2	60	7.5
TSS	<10	<10		7.5
TKN	7.4	4.1	45	n/a
TAN	6.1	3.1	49	2.0 (Fall, Spring) 4.0 (Winter)
NO <sub>2</sub>	0.123	0.355	(189%)	n/a
NO <sub>3</sub>	0.31	1.09	(252%)	n/a
TP	0.20	0.27	(35%)	0.7
dP	0.11	0.18	(64%)	n/a
Alkalinity	180	190	(6%)	n/a
рН	7.96	7.84	n/a	6.0 to 9.5
E. coli	20	<10	50%	100

#### Notes

- 1. Data used in the above table is from lab results from a single grab sample collected on Nov. 7, 2023.
- 2. dP is Dissolved Phosphorous

Conclusions from the above are limited since data is based on a single grab sample.

Comparing the Lagoon 3 effluent quality to filter effluent quality indicates that the filters are generally providing a reduction in BOD5, TAN, and E. coli. Lagoon effluent TSS was already low. TP increased but was low from the lagoon. Nitrite and nitrate increased through the filters, indicating some passive nitrification (oxidization of ammonia) occurring through the filters.

There was very little vegetation observed covering the filter surface area during our site visit. The filter feed piping appeared in good condition with a few sections of piping and pipe supports that had settled. Seasonal operation of the filters had just ended prior to our site visit on November 7, 2023. Consequently, the effluent spray pattern and distribution could not be observed. Ideally, effluent should be evenly distributed across the filter media surface while the filter pump station pumps were operating. It was noted that 3 of the 4 filter cells (Cells 1, 2, and 4) had standing water on the surface even though the last dosing period was the previous day, indicated poor drainage through the filter media.



Photos from the inspection are shown below.



Figure 6a: Filter Cell 3 (facing north)



Figure 6c: Filter Cell 2 (facing north)



Figure 6b: Filter Cell 1 (facing north)



Figure 6d: Filter sand test pit

# 5.3 Filter Media Analysis

Five (5) filter media samples were collected from Filter Cell 3 (northwest cell). Two sand samples were collected from a depth of approx. 300mm below the surface ("shallow"), and two from approx. 600mm below the surface ("deep") in the north and south areas of Cell 1. A fifth sample was collected of the surface layer. Each sample was analyzed for grain size distribution, soil classification, percolation rate, coefficient of uniformity (Cu), effective grain size (D-10), and grain size distribution curve. Results from the soil testing are summarized in the following Table with the analytical report from the testing laboratory is included in Appendix C.

March 2024



Table 7. Summary of Filter Media Sand Analysis

Sample ID/ Parameter	Cell 3 Surface	Cell 3 North - Shallow	Cell 3 North - Deep	Cell 3 South - Shallow	Cell 3 South - Deep
Grain Size Distribution and Soil Classification	SAND, trace silt, trace clay, trace gravel (SP)	SAND, trace silt, trace clay, trace gravel (SP)	SAND, trace silt, trace gravel (SW)	SAND, trace silt, trace gravel (SP)	SAND, trace silt, trace gravel, trace clay (SW)
Percolation Rate (min/cm)	2-8	2-8	2-12	2-8	2-12
Percolation Rate (L/m2/d)	1,800 – 7,200	1,800 – 7,200	1,200 – 7,200	1,800 – 7,200	1,200 – 7,200
Coefficient of Uniformity (Cu)	7.75	6.04	5.28	6.96	5.08
Effective Grain Size (D-10)	0.08 mm	0.14 mm	0.15 mm	0.11 mm	0.16 mm

Definition of the terms in the above table are as follows.

<u>Soil classification</u>: A general description of the physical properties of a soil sample in terms of its grain size and grain size distribution.

<u>Percolation rate</u>: Expressed in minutes per centimetre (min/cm). A higher percolation rate applies to more porous soils with coarser grain size.

<u>Coefficient of Uniformity (Cu)</u>: A dimensionless parameter. A higher Coefficient of Uniformity corresponds to a lower degree of uniformity of the media particles sizes.

<u>Effective Grain Size (D-10)</u>: Expressed in units of millimetres (mm). Grain size factor D-10 indicates that 10% of the particles are less than the specified particle size in the sample.

GMBP compared filter media sample results with MECP Design Guidelines. A summary table of the guidelines, comparison to field results, and GMBP recommendations are provided below:





Table 8. Summary of Literature Review and Comparison to Field Analysis

Parameter	MECP Guideline (12.3.6)	Clifford WWTP	Findings / Recommendations
Dosage	<ul> <li>Filter surface should be flooded with lagoon effluent 1-2 times per day.</li> <li>Filters should be capable of filtering total daily hydraulic load in &lt;6 hours to ensure maximum head and bed reaeration after drainage.</li> <li>Typical ISF filter run length is &lt;30 days, with lagoon effluent TSS &gt;50 mg/L.</li> <li>Hydraulic loading rate of 500 L/(m²-d).</li> </ul>	<ul> <li>Operations confirmed that filters are dosed through 8 cycles/day. Each cycle consists of 15 min dose, 45 min. rest period.</li> <li>Hydraulic loading rate based on current operation is 470 L/(m²-d), within MECP Guidelines.</li> </ul>	<ul> <li>Dosage is within MECP guidelines.</li> <li>Site observations indicate filters beds are draining very slowly, impairs filtration hydraulic capacity</li> <li>Hydraulic overloading is not occurring, not an issue.</li> </ul>
Bed Depth and Condition	<ul> <li>Sand depth should be 900mm and underlain by graded gravel layer.</li> <li>Allow removal of top 2-5cm layer during each cleaning cycle and replacement of that sand about once per year.</li> </ul>	<ul> <li>Original design filter media depth was 760mm</li> <li>Current bed depth is approx. 475 mm to 550mm, with an underlain gravel layer.</li> <li>300mm of sand media was removed in 2023 but not replenished</li> </ul>	<ul> <li>Sand media depth has become too shallow. Directly impacts filtration performance.</li> <li>Original sand media is still in place since 1995. Remove original sand media and replace with new sand.</li> </ul>
Effective Particle Size (D10)	• 0.15 to 0.30mm	• 0.11 – 0.16 mm	Trending low, i.e. media grain size is too fine. Replace removed sand with effective particle sizes closer to 0.20 to 0.30mm.
Uniformity Coefficient (Cu)	• <5	• 5 to 7	Grain size distribution too broad, well graded.

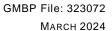
Clifford WWTP results were also compared to additional key design guidelines and research papers related to design and operation of ISF's for filtration of secondary wastewater effluent. A summary of results in shown on the next page in Table 9.

MARCH 2024



Table 9. Summary of Literature Review on ISFs

Reference	Hydraulio Rates (m	Loading ³/m²/day)	Filter Media Properties		
	Min	Max	Description	D10 (mm)	
Clifford WWTP Design Basis	0.47	0.47	Effective D10 particle size = 0.2 mm, Cu < 3	0.20	
US EPA Fact Sheet (1999)	0.08	0.20	Washed durable granular material, Cu < 4.0	0.25 to 0.75	
US EPA Fact Sheet (2002)	0.04	0.08	Washed durable sand with rounded grains, Cu<4.0	0.25 to 1.00	
British Columbia – Ministry of Environment	no data	no data	No data	no data	
National Small Flows Clearinghouse – Fact Sheet	no data	no data	No data	no data	
Orenco Intermittent Sand Filters	no data	no data	No data	no data	
Utah State University – 1974	0.03	0.12	Well graded sand, insoluble, no clay or loam, Cu<5.0	0.2 to 0.5	
Utah State University – 1977	1.40	2.80	Well graded sand, insoluble, no clay or loam, Cu<5.1	0.2 to 0.6	
Washington State – Dept. of Health (2012)	0.03	0.04	Coarse D10 >0.30, Fine ASTM C-33 D10 ~0.15	0.15	
Water Environment Federation Article (1961)	0.08	0.16	Washed sand, free from organics, Cu < 4	0.3 to 0.5	
Ministry of Works and Development (New Zealand)	0.50	1.50	Washed sand, free from organics, Cu < 4	0.3 to 0.6	
Univ. of Nevada "Upgrading Pond Effluents – An Overview" (1995)	0.37	0.56	Selected sand, Cu < 7	0.2 to 0.3	
MOEE Paper "Upgrading Effluent Quality for Lagoon-Based Systems " (1995)	0.40	0.50			
Averages from Reference Literature	0.34	0.64		0.2 to 0.6	
Clifford WWTP	1.2	7.2	Sand, trace silt, clay, and gravel (SP, SW), avg. Cu = 5.8	0.11 to 0.16	





Analysis of the filter sand media reveals that the percolation rate was within accepted design values. However, the Effective Particle Size ranges from 0.11 to 0.16mm, is at the low end of MECP Guidelines of 0.15 to 0.30mm and is lower than most of the examples in the literary review. Also, the uniformity coefficient (Cu) was above design guidelines, meaning the grain size distribution is too varied, not sufficiently uniform. This result could be explained by long-term accumulation of fine particles from lagoon effluent in the upper layer of the media bed.

The following table summarizes calculations on the loading rate being applied to the sand filters based on filter dimensions and the capacity of the lagoon effluent pumps that dose the filters.

**Table 10. Filter Media Loading Analysis** 

Filter Bed Dimensions			Flow	Applicable	Calc'd Loading	
Length (m)	Width (m) No. of Cells Area (sq.m.)		Rate (L/s)	Timing	Rate (m3/m2/d)	
Based on the pumping rate from the Lagoon Effluent Pumpir				g Station		
23	20	4	1,840	30	2 hrs/day	470
Based on the rated hydraulic capacity of the plant at 500 m3/day						
23	20	4	1,840	5.8	6 months/yr	543

MECP Design /Guidelines indicate the hydraulic loading rate to intermittent slow sand filters should not exceed 500 m3/m3/day.





March 2024

#### 6. CONCLUSIONS AND RECOMMENDATIONS

### 6.1 System Capacity

Analysis of system flows indicates that the system consistently operates well below rated hydraulic capacity on average at approx. 50% of plant rated capacity. Consequently, declining final effluent quality is not due to increasing flows to the plant.

### 6.2 Raw Sewage Monitoring

Analysis of plant operating data indicates that the raw sewage concentration for key wastewater parameters, including BOD<sub>5</sub>, TSS, TP, and TAN are increasing for the period from January 1, 2020 to September 30, 2023. This may partly explain the corresponding recent decline in final effluent quality for those parameters.

Increasing raw sewage strength may be due to implementation of water conservation measures such as low-flow plumbing fixtures, or a reduction in extraneous flows (inflow and infiltration) into the sanitary sewer collection system.

# 6.3 Facultative Lagoons

The MECP Design Guidelines for Sewage Works defines lagoons as equivalent to secondary treatment capable of achieving effluent CBOD<sub>5</sub> concentrations less than 25 mg/L, TSS concentrations less than 30 mg/L, and TP concentrations less than 1.0 mg/L. Based on a single gab sample collected on November 7, 2023, the lagoons appear to be functioning well with lagoon effluent quality exceeding MECP Design Guidelines.

The calculated organic loading rate to the lagoons appears to be within MECP Design Guidelines, although limited operational data was available to confirm this. Also, the measured depth of sludge blanket in Lagoon Cells 1 and 2 were determined to be low in 2018. Consequently, there was no evidence to suggest that declining final effluent quality is attributable to performance or available capacity of the lagoons.

### 6.4 Intermittent Sand Filters

### **Filter Maintenance**

It is recommended that vegetation be completely removed from the filter bed, including any roots or cuttings, each operating season. The above-ground irrigation piping should be temporarily removed to allow the surface of the sand media to be ploughed or raked with a small farm tractor to restore some of the lost filtration capacity. The irrigation piping would be re-installed after this work. This process should be repeated as needed to maintain the filter cells free of vegetation. Based on discussions with plant operations staff, the filters are being properly maintained.

#### **Filter Operation**

The filter loading rates and dosage timing are considered to be acceptable and in line with common practice for intermittent slow sand filters. No changes to the operation of the filters are recommended at this time.

#### Partial Filter Sand Replacement (Top 300mm Layer)

The thickness of the sand media in the filter beds is less than adequate based on the original design of the filters, MECP Design Guidelines, and literature review. Each filter bed could be upgraded by adding a 300mm layer of new sand media of the appropriate particle size, grain size distribution, and other key media characteristics as outlined above. That layer would be added on top of the existing filter sand.

For each consecutive year, 5cm of sand can be removed, cleaned, and reused, until the effective particle size diminishes, and new sand should be placed. There is no readily available system or company that has a means to wash and regrade filter sand media in situ for ISSF's.

Through the results of previous informal surveys of other municipalities in Southern Ontario with lagoons and ISSF's, there is limited success with partial sand replacement (Blenheim WWTP & Alymer WWTP). This feedback is supported by literature review stating that the fouling of filter beds typical occurs in the upper layers between 100 mm to 300mm.



**MARCH 2024** 

Sand media samples were taken on-site on November 7, 2023, and results showed that the effective grain size has been reduced since the original installation in both the shallow bed and deep bed sample locations, indicating that fines may have mitigated into the lower portion of the filter media. Therefore, partial sand media replacement may not fully restore the filter basins to their original performance if fines have migrated through the entire sand media profile.

#### **Total Filter Sand Replacement**

The sand filters have been in operation for 28 years to year 2023. These types of filters have no means for regular media backwashing and regeneration like engineered backwashing media filters. Full sand filter depth replacement was considered as a conservative approach to rehabilitation of the existing filters. Grain size analysis from samples taken from shallow and deep layers of the sand media indicated a finer material than recommended by MECP Guidelines and compared to reference literature. This could be explained by long-term migration of fines from the upper portions of the filter media to the lower portions of the bed. In addition, filter bed thickness is less than adequate since removal of the top 300mm layer in 2023.

#### **Construction of Additional Filter Cell**

There is physical space available on plant property to construct additional filter cell(s) adjacent to the existing filter cells on the north side. Results of our assessment indicate that constructing an additional (i.e. fifth) filter cell adjacent to the existing filter cells is not considered necessary or the most viable option for system upgrades at this time.

#### 7. PRELIMINARY COST ESTIMATES FOR RECOMMENDATIONS

The following table provides preliminary cost estimates for each recommendation in order of ascending cost. It may be advantageous to complete upgrades in a phased approach until filtered effluent quality has improved. A detailed breakdown for each cost estimate is contained in Appendix D.

Table 11. Summary of Cost Estimates for Upgrade Recommendations

Option No.	Description of Upgrades	Cost Estimate (\$ 2024, excl. HST)
1.	<ul> <li>Partial sand media replacement</li> <li>Remove and stockpile irrigation piping, supports, and fittings</li> <li>300 mm sand layer across 1,840 m² surface area = 550m³ total sand media volume.</li> <li>Re-install irrigation piping, supports, and fittings</li> <li>Repair or replace any deficient pipe supports and sections of irrigation piping</li> </ul>	\$84,000
2.	<ul> <li>Full sand media replacement</li> <li>Remove and stockpile irrigation piping, supports, and fittings</li> <li>760mm sand layer across 1,840 m² surface area = 1,400 m³ total sand media volume.</li> <li>Re-install irrigation piping, supports, and fittings</li> <li>Repair or replace any deficient pipe supports and sections of irrigation piping</li> </ul>	\$204,000

#### Notes

- 1. Rough budget estimate for \$30 to \$35/tonne. Delivery charge may be ~ \$6/t/hour
- 2. Density of uncompacted filter sand is 2.0 tonne/cu.m.
- 3. Budget estimates in the above table do not include HST.



The 2 key recommendations above are to either replace the top 300mm of filter sand, or replace the entire layer of filter sand. Based on assessment of the performance of the existing sand filters and geotechnical analysis of the sand filter media, it is recommended to proceed with full replacement of the sand filter media for each filter cell. Based on sand media grain size analysis, it is likely that the entire depth of the sand filter media may be compromised due to long-term infiltration of fines, and consequently, it is recommended to replace the entire depth of filter sand to restore filtration performance. Although the cost for replacement of the full depth of filter sand is approx. 2.5 times the cost to replace only the top 300mm of filter media, the full life-cycle cost benefit supports full sand replacement.

Figure 7 below presents a cross-section of the existing filter bed layers, indicating replacement of the sand filter media to its entire depth, but leaving the gravel support layers intact.

Description of Media	Symbol	Thickness	Status
Filter Sand Effective Grain Size : 0.8 to 1.2mm D-10 size = 0.2mm Uniformity Coefficient less than 5.0		760mm	To Be Replaced
5mm Clear Stone		75mm	To Remain
9.5mm Clear Stone		75mm	To Remain
13mm Clear Stone		75mm	To Remain
19mm Clear Stone		75mm	To Remain
19mm Clear Stone 100mm Perforated Underdrain Pipes	0 0	275mm	To Remain

Figure 7: Filter Bed Cross-Section

Yours truly,

**GM BLUEPLAN ENGINEERING LIMITED** 

Just Partition

Per:

Grant Parkinson, P. Eng.

**Project Manager** 

Encl.

APPENDIX A:
MECP ENVIRONMENTAL COMPLIANCE APPROVAL

# **Content Copy Of Original**



Ministry of the Environment and Climate Change Ministère de l'Environnement et de l'Action en matière de changement climatique

# AMENDED ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 8901-A8YJ9C Issue Date: June 7, 2016

The Corporation of the Town of Minto 5941 Highway 89 R.R. 1, P.O. Box 160, Harriston Minto, Ontario

N0G 1Z0

Site Location: Clifford Wastewater Treatment Plant

Lot 57 and 58, Concession C

Town of Minto, County of Wellington

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

Existing sewage works at the Clifford Wastewater Treatment Plant with a rated average daily flow capacity of 500 m 3 /d (peak flow capacity of 2,151 m 3 /d), located on Part of Lots 57 and 58, Concession C, in the Town of Minto, discharging to the Coon Creek via the Minto Municipal Drain No. 93, consisting of the following:

# **Sanitary Sewer**

- Sanitary sewer on James Street from Clark Street to Taylor Street, on Queen Street from Ann Street to Brown Street, on Geddes Street from approximately 200 m west of Cecelia Street to William Street, on Allan Street from Ann Street to Brown Street and from approximately 190 m west of Cecelia Street to approximately 120 m east of Minto Street, on John Street from Ann Street to Elora Street and from Brown Street to Taylor Street, on Nelson Street from Ann Street to Brown Street, on Main Street from Ann Street to Elora Street and from William Street to Brown Street, on Taylor Street from John Street to James Street, on Brown Street from Nelson Street to James Street, on Clark Street from Main Street to James Street, on Elora Street from Nelson Street to James Street, on Minto Street from Allan Street to Geddes Street, and Easement from intersection of James Street and Taylor Street to approximately 50 m west of James Street;

# **Influent Pumping Station**

- a 3 m diameter raw sewage pumping station, equipped with two (2) submersible pumps, each rated at 29 L/s at 15 m TDH and a screening basket and above ground hoisting equipment, together with two (2) 200 mm diameter forcemains from the pumping station to the control building, thence via a 200 mm diameter forcemain to the aerated facultative lagoon.

## **Control Building**

A 10.8 m x 15.8 m single story control building, housing the following:

- two (2) rotary positive displacement air blowers (one standby), each capable of providing 99 L/s of air at 41 kPa;

- one (1) chemical feed system consisting of two (2) chemical metering pumps (one standby), each capable of feeding up to a maximum of 38 L/hr of alum solution and chemical storage of approximately 27,000 L in PVC closed liner within spill containment area, with eyewash and deluge;
- a 60 kw diesel engine generator set and a 908 L capacity fuel storage tank within spill containment area;

# **Aerated Facultative Lagoon**

- a 3.6 m deep (with 0.6 m freeboard) lined aerated lagoon with a surface area of approximately 1,420 m 2, and liquid depth of 3 m, providing a total storage volume of 2,000 m 3, equipped with coarse air bubble diffusion system and inlet and outlet chambers, discharging to Facultative Lagoon No.1 as described below:
- one (1) bypass channel between the downstream of influent pumping station and the inlet of Facultative Lagoon No.1 for bypassing the Aerated Facultative Lagoon;

# **Facultative Lagoons**

- three (3) facultative lagoon cells operating in series, Lagoons No. 1 and No. 2 with a depth of 1.8 m and Lagoon No. 3 having a depth of 2.4 m, providing a total storage capacity of 150,000 m 3, with 4:1 interior side slope and 3:1 exterior side slope and inlet and outlet chamber, discharging effluent from facultative lagoon No.3 to the Intermittent Sand Filter via the Treated Effluent Pumping Station as described below;

# **Treated Effluent Pumping Station**

- a 3.2 m x 8.2 m underground concrete pumping chamber located within the embankment of facultative lagoon No. 3, equipped with four (4) submersible sewage pumps, each rated at 30 L/s at 5.25 m TDH with 200 mm diameter forcemain to discharge effluent from Facultative Lagoon No.3 to the Intermittent Sand Filter;

### **Intermittent Sand Filter**

- an intermittent sand filter made up of four (4) cells, each with a filtration surface area of approximately 460 m 2, with filter media consisting of a 300 mm layer of graded crushed stone overlain by a 760 mm layer of sand, a perforated aluminum distribution piping system and underdain system with 0.6 m freeboard;
- one (1) bypass channel between the downstream of Facultative Lagoon No. 3 and the inlet of the Filtered Effluent Storage Lagoon No. 1 for bypassing the Intermittent Sand Filter;

# **Filtered Effluent Storage lagoons**

- two (2) filtered effluent storage lagoons (No.1 and No.2), each with a surface area of approximately 42,900 m 2 and liquid depth of 2.4 m, providing a total storage capacity of 187,600 m 3 with inlet and outlet chambers with discharge to the open outfall ditch via the control building;

# **Effluent Outfall**

- one (1) 200 mm diameter outfall sewer connected to an open ditch, discharging to Coon Creek via the Minto Municipal Drain No. 93;

all other controls, piping, valves, drains, and appurtenances essential for the proper operation of the aforementioned sewage works;

all in accordance with the submitted supporting documents listed in Schedule A.

For the purpose of this environmental compliance approval, the following definitions apply:

"Act" means the Ontario Water Resources Act, R.S.O. 1990, Chapter 0.40, as amended;

"Approval" means this entire document and any schedules attached to it, and the application;

"Average Daily Flow" means the cumulative total sewage flow to the sewage works during a calendar year divided by the number of days during which sewage was flowing to the sewage works that year:

"BOD5" (also known as TBOD 5 ) means five day biochemical oxygen demand measured in an unfiltered sample and includes carbonaceous and nitrogenous oxygen demand;

"By-pass" means diversion of sewage around one or more unit processes within the Sewage Treatment Plant with the diverted sewage flows being returned to the Sewage Treatment Plant treatment train upstream of the Final Effluent sampling location, and discharging to the environment through the Sewage Treatment Plant outfall;

"CBOD5" means five day carbonaceous (nitrification inhibited) biochemical oxygen demand measured in an unfiltered sample;

"Daily Concentration" means the concentration of a contaminant in the effluent discharged over any single day, as measured by a composite or grab sample, whichever is required;

"Director" means a person appointed by the Minister pursuant to section 5 of the EPA for the purposes of Part II.1 of the EPA:

" E. Coli" refers to the thermally tolerant forms of Escherichia that can survive at 44.5 degrees Celsius;

"Emergency Situation" means a structural, mechanical or electrical failure that causes a temporary reduction in the capacity of the Sewage Treatment Plant or an unforeseen flow condition that may result in:

- a) danger to the health or safety of any person; or,
- b) injury or damage to any property, or serious risk of injury or damage to any property; or
- c) treatment process biomass washout. "EPA" means the *Environmental Protection Act*, R.S.O. 1990, c.E.19, as amended;

"Equivalent Equipment" means a substituted equipment or like-for-like equipment that meets the required quality and performance standards of a named equipment;

"Event" means an action or occurrence, at a given location within the Sewage Treatment Plant that causes a Plant Bypass or Plant Overflow. An Event ends when there is no recurrence of a Bypass or Overflow in the 12-hour period following the last Bypass or Overflow. Two Events are separated by at least 12 hours during which there has been no recurrence of a Bypass or Overflow;

"Final Effluent" means sewage discharge via the Sewage Treatment Plant outfall after undergoing the full train of unit processes as listed in the Approval;

"Geometric Mean Density" is the nth root of the product of multiplication of the results of n number of

samples over the period specified;

"Limited Operational Flexibility" (LOF) means any modifications that the Owner is permitted to make to the Works under this Approval;

"Ministry" means the ministry of the government of Ontario responsible for the EPA and OWRA and includes all officials, employees or other persons acting on its behalf;

"Monthly Average Concentration" means the arithmetic mean of all Daily Concentrations of a contaminant in the effluent sampled or measured, or both, during a calendar month;

"Monthly Average Daily Flow" means the cumulative total sewage flow to the sewage works during a calendar month divided by the number of days during which sewage was flowing to the sewage works that month:

"Notice of Modifications" means the form entitled "Notice of Modifications to Sewage Works";

"Owner" means The Corporation of the Town of Minto and its successors and assignees;

"OWRA" means the *Ontario Water Resources Act*, R.S.O. 1990, c. O.40, as amended;

"Peak Flow Rate" means the maximum rate of sewage flow for which the plant or process unit was designed;

"Plant Overflow" means a discharge to the environment from the Sewage Treatment Plant at a location other than the plant outfall or into the plant outfall downstream of the Final Effluent sampling location;

"Rated Capacity" means the Average Daily Flow for which the Works are approved to handle;

"Regional Water Compliance Manager" means the Regional Water Compliance Manager of the West Central Region of the Ministry;

"Sewage Treatment Plant" means the entire sewage treatment and effluent discharge facility;

"Water Supervisor" means the Water Supervisor for the Guelph office of the Ministry; and

"Works" means the sewage works described in the Owner's application, and this Approval, and includes Proposed Works, Previous Works, and modifications made under Limited Operational Flexibility.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

### TERMS AND CONDITIONS

### 1. GENERAL PROVISIONS

- (1) The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the conditions herein and shall take all reasonable measures to ensure any such person complies with the same.
- (2) Except as otherwise provided by these conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application

for approval of the Works.

- (3) Where there is a conflict between a provision of any document in the schedule referred to in this Approval and the conditions of this Approval, the Conditions in this Approval shall take precedence, and where there is a conflict between the documents in the schedule, the document bearing the most recent date shall prevail.
- (4) Where there is a conflict between the documents listed in the Schedule submitted documents, and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.
- (5) The Conditions of this Approval are severable. If any Condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such condition to other circumstances and the remainder of this Approval shall not be affected thereby.
- (6) The approval granted by this Approval is based upon a review of the Works in the context of its effect on the environment, its process performance and general principles of wastewater engineering. The review did not include a consideration of the architectural, mechanical, electrical or structural components and minor details of the Works except to the extent necessary to review the Works.

#### 2. EXPIRY OF APPROVAL

The approval issued by this Approval will cease to apply to those parts of the Works which have not been constructed within five (5) years of the date of this Approval.

### 3. CHANGE OF OWNER

- (1) The Owner shall notify the Water Supervisor and the Director, in writing, of any of the following changes within thirty (30) days of the change occurring:
- (a) change of Owner;
  - (b) change of address of the Owner;
  - (c) change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the *Business Names Act*, R.S.O. 1990, c.B17 shall be included in the notification to the Water Supervisor;
  - (d) change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the *Corporations Information Act*, R.S.O. 1990, c. C39 shall be included in the notification to the Water Supervisor;
- (2) In the event of any change in ownership of the Works, other than a change to a successor municipality, the Owner shall notify in writing the succeeding owner of the existence of this Approval, and a copy of such notice shall be forwarded to the Water Supervisor and the Director.

## 4. BY-PASSES

(1) Any By-pass or Plant Overflow is prohibited, except:

- (a) in an Emergency Situation; (b) where the By-pass / Plant Overflow is a direct and unavoidable result of a planned maintenance procedure, the Owner notified the Water Supervisor 15 days prior to the By-pass and the Water Supervisor has given written consent of the Bypass; and (c) where the By-pass / Plant Overflow is planned for research or training purposes, the discharger notified the Water Supervisor 15 days prior to the By-pass / Plant Overflow and the Water Supervisor has given written consent of the By-pass / Plant Overflow. (2) The Owner shall forthwith notify the Spills Action Centre (SAC) and the Medical Officer of Health of all By-pass and Plant Overflow Events. This notice shall include, at a minimum, the following information: (a) the date, time, and duration of the Event;
- - (b) the location of the Event;
  - (c) the measured or estimated volume of the Event (unless the Event is ongoing);
  - (d) the reason for the Event; and
  - (e) the level of treatment the By-pass(es) and/or Plant Overflow(s) received and disinfection status of same.
- (3) The Owner shall submit By-pass and Plant Overflow Event Reports to the Ministry's local office on a quarterly basis, no later than each of the following dates for each calendar year: February 14, May 15, August 14, and November 15. Event Reports shall be in an electronic format specified by the Ministry. In each Event Report the Owner shall include, at a minimum, the following information on any Events that occurred during the preceding quarter:
- (a) the date of the Event(s);
  - (b) the measured or estimated volume of the Event(s);
  - (c) the duration of the Event(s);
  - (d) the location of the Event(s);
  - (e) the reason for the Event(s); and
  - (f) the level of treatment the By-pass(es) and/or Plant Overflow(s) received and disinfection status of same.
- (4) The Owner shall use best efforts to collect a representative sample consisting of a minimum of two
- (2) grab samples of the By-pass / Plant Overflow and have it analyzed for parameters outlined in

Condition 6, one at the beginning of the Event and the second approximately near the end of the Event, to best reflect the effluent quality of such By-pass or Plant Overflow.

(5) The Owner shall maintain a logbook of all Plant By-passes and Plant Overflows, which shall contain, at a minimum, the types of information set out in subsection 2 (a) to 2(e) in respect of each By-pass and Plant Overflow.

### 5. EFFLUENT OBJECTIVES

(1) The Owner shall use best efforts to operate and maintain the Works with the objective that the concentrations of the materials named below as effluent parameters are not exceeded in the effluent from the Works.

Table 1 - Effluent Objectives		
Effluent Parameter	Parameter Concentration Objective	
	(milligrams per litre unless otherwise	
	indicated)	
CBOD5	7.5	
Total Suspended Solids	7.5	
Total Phosphorus	0.7	
Total Ammonia Nitrogen	2.0	
- March, April, November	4.0	
- December, January, February		
Dissolved Oxygen	no less than 5.0	
E. coli	100 organisms per 100 millilitres	
	Monthly Geometric Mean Density	

- (2) The Owner shall use best efforts to:
  - (a) maintain the pH of the effluent from the Works within the range of 6.0 9.5, inclusive, at all times;
  - (b) operate the works within the Rated Capacity of the Works;
  - (c) ensure that the effluent from the Works is essentially free of floating and settleable solids and does not contain oil or any other substance in amounts sufficient to create a visible film or sheen or foam or discolouration on the receiving waters.
- (3) The Owner shall include in all reports submitted in accordance with Condition 10 a summary of the efforts made and results achieved under this Condition.

### 6. EFFLUENT LIMITS

(1) The Owner shall operate and maintain the Works such that the concentrations and waste loadings of the materials named below as effluent parameters are not exceeded in the effluent from the Works.

Table 2 - Effluent Limits	
Effluent Parameter	Monthly Average Concentration
	(milligrams per litre unless otherwise
	indicated)

Column 1	Column 2
CBOD5	12.0
Total Suspended Solids	12.0
Total Phosphorus	0.7
Total Ammonia Nitrogen	3.0
- March, April, November	6.0
- December, January, February	

- (2) For the purposes of determining compliance with and enforcing subsection (1):
  - (a) The Monthly Average Concentration of a parameter named in Column 1 of subsection (1) shall not exceed the corresponding maximum concentration set out in Column 2 of subsection (1).
- (3) The Owner shall design and construct and operate and maintain the Works such that the pH of the effluent is maintained within the range of 6.0 9.5, inclusive, at all times.
- (4) Paragraphs (a) of subsection (2) shall apply upon the issuance of this Approval.
- (5) The effluent limit set out in subsection (3) shall apply upon the issuance of this Approval.

# 7. SPECIAL CONDITIONS

(1) The treated effluent discharge from the filtered effluent storage lagoons to the Minto Municipal Drain No. 93 shall not exceed the allowance discharge as follows:

Table 3 - Effluent Discharge Flows			
Month	Allowable Discharge (m 3 /d)		
November	470		
December	734		
January	1,028		
February	1,075		
March	1,970		
April	1,345		
May to October (inclusive)	0		

### 8. OPERATION AND MAINTENANCE

- (1) The Owner shall exercise due diligence in ensuring that, at all times, the Works and the related equipment and appurtenances used to achieve compliance with this Approval are properly operated and maintained. Proper operation and maintenance shall include effective performance, adequate funding, adequate operator staffing and training, including training in all procedures and other requirements of this Approval and the Act and regulations, adequate laboratory facilities, process controls and alarms and the use of process chemicals and other substances used in the Works.
- (2) The Owner shall prepare an operations manual upon the issuance of this Approval, that includes, but not necessarily limited to, the following information:
  - (a) operating procedures for routine operation of the Works;
  - (b) inspection programs, including frequency of inspection, for the Works and the methods or tests employed to detect when maintenance is necessary;

- (c) repair and maintenance programs, including the frequency of repair and maintenance for the Works:
- (d) procedures for the inspection and calibration of monitoring equipment;
- (e) a spill prevention control and countermeasures plan, consisting of contingency plans and procedures for dealing with equipment breakdowns, potential spills and any other abnormal situations, including notification of the Water Supervisor; and
- (f) procedures for receiving, responding and recording public complaints, including recording any followup actions taken.
- (3) The Owner shall keep a complete set of the up-to-date record drawings at the site of the sewage works throughout the operational life of the sewage works, and upon request, shall make the drawings available for inspection by Ministry staff.
- (4) The Owner shall maintain the operations manual current and retain a copy at the location of the Works for the operational life of the Works. Upon request, the Owner shall make the manual available to Ministry staff.
- (5) The Owner shall provide for the overall operation of the Works with an operator who holds a licence that is applicable to that type of facility and that is of the same class as or higher than the class of the facility in accordance with Ontario Regulation 129/04.

# 9. MONITORING AND RECORDING

The Owner shall, upon commencement of operation of the Works, carry out the following monitoring program:

- (1) All samples and measurements taken for the purposes of this Approval are to be taken at a time and in a location characteristic of the quality and quantity of the effluent stream over the time period being monitored.
- (2) For the purposes of this condition, the following definitions apply:
  - (a) Weekly means once every week;
- (3) Samples shall be collected at the following sampling points, at the minimum frequency specified, by means of the specified sample type and analyzed for each parameter listed and all results recorded:

Table 4a - Raw Sewage Monitoring			
(Sampling point at the downstream of influent pumping station)			
Parameters	Sample Type	Frequency	
BOD5	Grab	weekly	
Total Suspended Solids	Grab	weekly	
Total Phosphorus	Grab	weekly	
Total Kjeldahl Nitrogen	Grab	weekly	

Table 4b - Effluent Monitoring	

(Sampling point at the do	wnstream of the Filtered Efflue	nt Storage Lagoon No.2)
Parameters	Sample Type	Frequency
CBOD5	at least 8-hour Composite	weekly
Total Suspended Solids	at least 8-hour Composite	weekly
Total Phosphorus	at least 8-hour Composite	weekly
Total Ammonia (Ammonia + Ammonium) Nitrogen	at least 8-hour Composite	weekly
E. Coli	Grab	weekly
Dissolved Oxygen	Grab	weekly
рН	Grab/Probe	weekly
Temperature	Grab/Probe	weekly

- (4) The methods and protocols for sampling, analysis and recording shall conform, in order of precedence, to the methods and protocols specified in the following:
  - (a) the Ministry's Procedure F-10-1, "Procedures for Sampling and Analysis Requirements for Municipal and Private Sewage Treatment Works (Liquid Waste Streams Only), as amended from time to time by more recently published editions;
  - (b) the Ministry's publication "Protocol for the Sampling and Analysis of Industrial/Municipal Wastewater" (January 1999), ISBN 0-7778-1880-9, as amended from time to time by more recently published editions;
  - (c) the publication "Standard Methods for the Examination of Water and Wastewater" (21st edition), as amended from time to time by more recently published editions;
- (5) The temperature and pH of the effluent from the Works shall be determined in the field at the time of sampling for Total Ammonia Nitrogen, using the methodology stipulated in "Ontario's Provincial Water Quality Objectives" dated July 1994, as amended.
- (6) The Owner shall install and maintain (a) continuous flow measuring device(s), to measure the flowrate of the influent to or effluent from the Works with an accuracy to within plus or minus 15 per cent (+/- 15%) of the actual flowrate for the entire design range of the flow measuring device, and record the flowrate at a daily frequency.
- (7) The Owner shall retain for a minimum of five (5) years from the date of their creation, all records and information related to or resulting from the monitoring activities required by this Approval.
- (8) Within 8 months of the issuance date of this Approval, the Owner shall submit a plan to the Water Supervisor for his approval outlining the periodic biological assessments to be undertaken (by the Owner) of the Coon Creek and for visual inspection of the Minto Municipal Drain No. 93 during the effluent discharge period. The plan shall include appropriate mitigative and contingency measures that will be undertaken in the event conditions in the Coon Creek and Minto Municipal Drain No. 93 deteriorate to unacceptable levels. Upon approval of this plan from the Water Supervisor, the plan

shall be implemented.

### 10. REPORTING

- (1) Fifteen (15) days prior to the date of a planned By-pass being conducted pursuant to Condition 4 and as soon as possible for an unplanned By-pass, the Owner shall notify the Water Supervisor (in writing) of the pending start date, in addition to an assessment of the potential adverse effects on the environment and the duration of the By-pass.
- (2) The Owner shall report to the Water Supervisor or designate, any exceedence of any parameter specified in Condition 6 orally, as soon as reasonably possible, and in writing within seven (7) days of the exceedence.
- (3) In addition to the obligations under Part X of the *Environmental Protection Act*, the Owner shall, within ten (10) working days of the occurrence of any reportable spill as defined in Ontario Regulation 675/98, bypass or loss of any product, by-product, intermediate product, oil, solvent, waste material or any other polluting substance into the environment, submit a full written report of the occurrence to the Water Supervisor describing the cause and discovery of the spill or loss, clean-up and recovery measures taken, preventative measures to be taken and schedule of implementation.
- (4) The Owner shall, upon request, make all manuals, plans, records, data, procedures and supporting documentation available to Ministry staff.
- (5) The Owner shall prepare and submit a performance report to the Water Supervisor on an annual basis, within ninety (90) days following the end of the period being reported upon. The first such report shall cover the first annual period following the commencement of operation of the Works and subsequent reports shall be submitted to cover successive annual periods following thereafter. The reports shall contain, but shall not be limited to, the following information:
  - (a) a summary and interpretation of all monitoring data and a comparison to the effluent limits outlined in Condition 6, including an overview of the success and adequacy of the Works;
  - (b) a description of any operating problems encountered and corrective actions taken;
  - (c) a summary of all maintenance carried out on any major structure, equipment, apparatus, mechanism or thing forming part of the Works;
  - (d) a summary of any effluent quality assurance or control measures undertaken in the reporting period;
  - (e) a summary of the calibration and maintenance carried out on all effluent monitoring equipment; and
  - (f) a description of efforts made and results achieved in meeting the Effluent Objectives of Condition 5.
  - (g) a tabulation of the volume of sludge generated in the reporting period, an outline of anticipated volumes to be generated in the next reporting period and a summary of the locations to where the sludge was disposed;

- (h) a summary of any complaints received during the reporting period and any steps taken to address the complaints;
- (i) a summary of all By-pass, spill or abnormal discharge events;
- (j) a copy of all Notice of Modifications submitted to the Water Supervisor as a result of Schedule B, Section 1, with a status report on the implementation of each modification;
- (k) a report summarizing all modifications completed as a result of Schedule B, Section 3; and
- (I) any other information the Water Supervisor requires from time to time.
- (6) The Owner shall, within thirty (30) calendar days of issuance of this Approval, submit a Municipal and Local Services Board Sewage Works Profile Information Form, and shall resubmit the updated document every time a notification is provided to the Water Supervisor in compliance with requirements of change of ownership under this Approval.

### 11. LIMITED OPERATIONAL FLEXIBILITY

- (1) The Owner may make modifications to the Works in accordance with the Terms and Conditions of this Approval and subject to the Ministry's "Limited Operational Flexibility Criteria for Modifications to Sewage Works", included under Schedule B of this Approval, as amended.
- (2) Sewage works proposed under Limited Operational Flexibility shall adhere to the design guidelines contained within the Ministry's publication "Design Guidelines for Sewage Works 2008", as amended.
- (3) The Owner shall ensure at all times, that the Works, related equipment and appurtenances which are installed or used to achieve compliance are operated in accordance with all Terms and Conditions of this Approval.
- (4) For greater certainty, the following are not permitted as part of Limited Operational Flexibility:
  - (a) Modifications to the Works that result in an increase of the Rated Capacity of the Works;
  - (b) Modifications to the Works that may adversely affect the approved effluent quality criteria or the location of the discharge/outfall;
  - (c) Modifications to the treatment process technology of the Works, or modifications that involve construction of new reactors (tanks) or alter the treatment train process design;
  - (d) Modifications to the Works approved under s.9 of the EPA, and
  - (e) Modifications to the Works pursuant to an order issued by the Ministry.
- (5) Implementation of Limited Operational Flexibility is not intended to be used for piecemeal measures that result in major alterations or expansions.
- (6) If the implementation of Limited Operational Flexibility requires changes to be made to the Emergency Response, Spill Reporting and Contingency Plan, the Owner shall, as deemed necessary in consultation with the Water Supervisor, provide a revised copy of this plan for approval to the local fire services authority prior to implementing Limited Operational Flexibility.

- (7) For greater certainty, any modification made under the Limited Operational Flexibility may only be carried out after other legal obligations have been complied with, including those arising from the Environmental Protection Act, Niagara Escarpment Planning and Development Act, Oak Ridges Moraine Conservation Act, Lake Simcoe Protection Act and Greenbelt Act.
- (8) Prior to implementing Limited Operational Flexibility, the Owner shall complete a Notice of Modifications describing any proposed modifications to the Works and submit it to the Water Supervisor.

### Schedule "A"

- 1. Amended Certificate of Approval for Municipal and Private Sewage Works No. 3-1547-92-947, issued on May 3, 1994.
- 2. All additional supporting documents provided by Mark Robertson from The Corporation of the Town of Minto.

# Schedule "B" Limited Operational Flexibility Criteria for Modifications to Sewage Works

1. The modifications to sewage works approved under an Environmental Compliance Approval (Approval) that are permitted under the Limited Operational Flexibility (LOF), are outlined below and are subject to the LOF conditions in the Approval, and require the submission of the Notice of Modifications. If there is a conflict between the sewage works listed below and the Terms and Conditions in the Approval, the Terms and Conditions in the Approval shall take precedence.

# 1.1 Sewage Pumping Stations

- a. Alter pumping capacity by adding or replacing equipment where new equipment is located within an existing sewage treatment plant site or an existing sewage pumping station site, provided that the modifications do not result in an increase of the sewage treatment plant Rated Capacity and the existing flow process and/or treatment train are maintained, as applicable.
- b. Forcemain relining and replacement with similar pipe size where the nominal diameter is not greater than 1,200 mm.

# 1.2 Sewage Treatment Process

- a. Installing additional chemical dosage equipment including replacing with alternative chemicals for pH adjustment or coagulants (non-toxic polymers) provided that there are no modifications of treatment processes or other modifications that may alter the intent of operations and may have negative impacts on the effluent quantity and quality.
- b. Expanding the buffer zone between a sanitary sewage lagoon facility or land treatment area and adjacent uses provided that the buffer zone is entirely on the proponent's land.
- c. Optimizing existing sanitary sewage lagoons with the purpose to increase efficiency of treatment operations provided that existing sewage treatment plant rated capacity is not exceeded and where no land acquisition is required.
- d. Optimizing existing sewage treatment plant equipment with the purpose to increase the

efficiency of the existing treatment operations, provided that there are no modifications to the works that result in an increase of the Rated Capacity, and may have adverse effects to the effluent quality or location of the discharge.

e. Replacement, refurbishment of previously approved equipment in whole or in part with Equivalent Equipment, like-for-like of different make and model, provided that the firm capacity, reliability, performance standard, level of quality and redundancy of the group of equipment is kept the same. For clarity purposes, the following equipment can be considered under this provision: pumps, screens, grit separators, blowers, aeration equipment, sludge thickeners, dewatering equipment, UV systems, chlorine contact equipment, bio-disks, and sludge digester systems.

# 1.3 Sewage Treatment Plant Outfall

a. Replacement of discharge pipe with similar pipe size and diffusers provided that the outfall location is not changed.

# 1.4 Sanitary Sewers

a. Pipe relining and replacement with similar pipe size within the Sewage Treatment Plant site, where the nominal diameter is not greater than 1,200 mm.

# 1.5 Pilot Systems

- a. Installation of pilot systems for new or existing technologies provided that:
  - i. any effluent from the pilot system is discharged to the inlet of the sewage treatment plant or hauled off-site for proper disposal,
  - ii. any effluent from the pilot system discharged to the inlet of the sewage treatment plant or sewage conveyance system does not significantly alter the composition/concentration of the influent sewage to be treated in the downstream process; and that it does not add any inhibiting substances to the downstream process, and
  - iii. the pilot system's duration does not exceed a maximum of two years; and a report with results is submitted to the Director and Water Supervisor three months after completion of the pilot project.
- 2. Sewage works that are exempt from section 53 of the OWRA by O. Reg. 525/98 continue to be exempt and are not required to follow the notification process under this Limited Operational Flexibility.
- 3. Normal or emergency operational modifications, such as repairs, reconstructions, or other improvements that are part of maintenance activities, including cleaning, renovations to existing approved sewage works equipment, provided that the modification is made with Equivalent Equipment, are considered pre-approved.
- 4. The modifications noted in section (3) above are not required to follow the notification protocols under Limited Operational Flexibility, provided that the number of pieces and description of the

equipment as described in the Approval does not change.

The reasons for the imposition of these terms and conditions are as follows:

- 1. Condition 1 is imposed to ensure that the Works are built and operated in the manner in which they were described for review and upon which approval was granted. This condition is also included to emphasize the precedence of Conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review. The condition also advises the Owners their responsibility to notify any person they authorized to carry out work pursuant to this Approval the existence of this Approval.
- 2. Condition 2 is included to ensure that the Works are constructed in a timely manner so that standards applicable at the time of Approval of the Works are still applicable at the time of construction, to ensure the ongoing protection of the environment.
- 3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to the approved works and to ensure that subsequent owners of the Works are made aware of the Approval and continue to operate the Works in compliance with it.
- 4. Condition 4 is included to indicate that By-pass / Plant Overflows of untreated or partially treated sewage to the receiving watercourse is prohibited, save in certain limited circumstances where the failure to By-pass / Plant Overflow could result in greater injury to the public interest than the Bypass itself where a By-pass / Plant Overflow will not violate the approved effluent requirements, or where the By-pass / Plant Overflow can be limited or otherwise mitigated by handling it in accordance with an approved contingency plan. The notification and documentation requirements allow the Ministry to take action in an informed manner and will ensure the Owner is aware of the extent and frequency of By-pass / Plant Overflow events.
- 5. Condition 5 is imposed to establish non-enforceable effluent quality objectives which the Owner is obligated to use best efforts to strive towards on an ongoing basis. These objectives are to be used as a mechanism to trigger corrective action proactively and voluntarily before environmental impairment occurs and before the compliance limits of Condition 6 are exceeded.
- 6. Conditions 6 and 7 are imposed to ensure that the effluent discharged from the Works meets the Ministry's effluent quality requirements thus minimizing environmental impact on the receiver and to protect water quality, fish and other aquatic life in the receiving water body.
- 7. Condition 8 is included to require that the Works be properly operated, maintained, funded, staffed and equipped such that the environment is protected and deterioration, loss, injury or damage to any person or property is prevented. As well, the inclusion of a comprehensive operations manual governing all significant areas of operation, maintenance and repair is prepared, implemented and kept up-to-date by the Owner and made available to the Ministry. Such a manual is an integral part of the operation of the Works. Its compilation and use should assist the Owner in staff training, in proper plant operation and in identifying and planning for contingencies during possible abnormal conditions. The manual will also act as a benchmark for Ministry staff when reviewing the Owner's operation of the Works.
- 8. Condition 9 is included to enable the Owner to evaluate and demonstrate the performance of the Works, on a continual basis, so that the Works are properly operated and maintained at a level which is consistent with the design objectives and effluent limits specified in the Approval and that the Works does not cause any impairment to the receiving watercourse.
- 9. Condition 10 is included to provide a performance record for future references, to ensure that the

Ministry is made aware of problems as they arise, and to provide a compliance record for all the terms and conditions outlined in this Approval, so that the Ministry can work with the Owner in resolving any problems in a timely manner.

10. Condition 11 is included to ensure that the Works are operated in accordance with the application and supporting documentation submitted by the Owner, and not in a manner which the Director has not been asked to consider. These Conditions are also included to ensure that a Professional Engineer has reviewed the proposed modifications and attests that the modifications are in line with that of Limited Operational Flexibility, and provide assurance that the proposed modifications comply with the Ministry's requirements stipulated in the Terms and Conditions of this Approval, MOE policies, guidelines, and industry engineering standards and best management practices.

Upon issuance of the environmental compliance approval, I hereby revoke Approval No(s). 3-1547-92-947 issued on May 3, 1994

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

- 1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
- 2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

Pursuant to subsection 139(3) of the Environmental Protection Act, a hearing may not be required with respect to any terms and conditions in this environmental compliance approval, if the terms and conditions are substantially the same as those contained in an approval that is amended or revoked by this environmental compliance approval.

The Notice should also include:

- 3. The name of the appellant;
- 4. The address of the appellant;
- 5. The environmental compliance approval number;
- 6. The date of the environmental compliance approval;
- 7. The name of the Director, and;
- 8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary\*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5

AND

The Director appointed for the purposes of Part II.1 of the Environmental Protection Act Ministry of the Environment and Climate Change 135 St. Clair Avenue West, 1st Floor Toronto, Ontario M4V 1P5

<sup>\*</sup> Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or

# www.ert.gov.on.ca

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 7th day of June, 2016

Fariha Pannu, P.Eng.
Director
appointed for the purposes of Part II.1 of
the *Environmental Protection Act* 

YZ/

c: DWMD Supervisor, MOECC Guelph Office Mark Robertson, The Corporation of the Town of Minto APPENDIX B: LAB RESULTS – WASTEWATER SAMPLING



Your P.O. #: 303072 Your Project #: 303072

Site Location: CLIFFORD WWTP Your C.O.C. #: 932020-03-01

**Attention: Kim Wilkinson** 

GM BluePlan Engineering Limited 650 Woodlawn Rd W Block C, Unit 2 Guelph, ON CANADA N1K 1B8

Report Date: 2023/11/15

Report #: R7913191 Version: 1 - Final

### **CERTIFICATE OF ANALYSIS**

BUREAU VERITAS JOB #: C3Y9188 Received: 2023/11/07, 15:53

Sample Matrix: Water # Samples Received: 2

		Date	Date		
Analyses	Quantity	Extracted	Analyzed	<b>Laboratory Method</b>	<b>Analytical Method</b>
Alkalinity	2	N/A	2023/11/09	CAM SOP-00448	SM 24 2320 B m
Biochemical Oxygen Demand (BOD)	2	2023/11/09	2023/11/14	CAM SOP-00427	SM 23 5210B m
Carbonaceous BOD	2	2023/11/09	2023/11/14	CAM SOP-00427	SM 23 5210B m
E.coli, (CFU/100mL)	2	N/A	2023/11/07	CAM SOP-00552	MECP E3371
Total Ammonia-N	2	N/A	2023/11/10	CAM SOP-00441	USGS I-2522-90 m
Nitrate & Nitrite as Nitrogen in Water (1)	2	N/A	2023/11/15	CAM SOP-00440	SM 23 4500-NO3I/NO2B
рН	2	2023/11/08	2023/11/09	CAM SOP-00413	SM 24th - 4500H+ B
Total Kjeldahl Nitrogen in Water	2	2023/11/09	2023/11/10	CAM SOP-00938	OMOE E3516 m
Total Phosphorus (Colourimetric)	1	2023/11/08	2023/11/09	CAM SOP-00407	SM 23 4500-P I
Total Phosphorus (Colourimetric)	1	2023/11/09	2023/11/10	CAM SOP-00407	SM 23 4500-P I
Dissolved Phosphorus	2	2023/11/14	2023/11/15	CAM SOP-00407	SM 23 4500-P I
Total Suspended Solids	1	2023/11/10	2023/11/11	CAM SOP-00428	SM 23 2540D m
Total Suspended Solids	1	2023/11/11	2023/11/13	CAM SOP-00428	SM 23 2540D m

#### **Remarks:**

Bureau Veritas is accredited to ISO/IEC 17025 for specific parameters on scopes of accreditation. Unless otherwise noted, procedures used by Bureau Veritas are based upon recognized Provincial, Federal or US method compendia such as CCME, MELCCFP, EPA, APHA.

All work recorded herein has been done in accordance with procedures and practices ordinarily exercised by professionals in Bureau Veritas' profession using accepted testing methodologies, quality assurance and quality control procedures (except where otherwise agreed by the client and Bureau Veritas in writing). All data is in statistical control and has met quality control and method performance criteria unless otherwise noted. All method blanks are reported; unless indicated otherwise, associated sample data are not blank corrected. Where applicable, unless otherwise noted, Measurement Uncertainty has not been accounted for when stating conformity to the referenced standard.

Bureau Veritas liability is limited to the actual cost of the requested analyses, unless otherwise agreed in writing. There is no other warranty expressed or implied. Bureau Veritas has been retained to provide analysis of samples provided by the Client using the testing methodology referenced in this report. Interpretation and use of test results are the sole responsibility of the Client and are not within the scope of services provided by Bureau Veritas, unless otherwise agreed in writing. Bureau Veritas is not responsible for the accuracy or any data impacts, that result from the information provided by the customer or their agent.

Solid sample results, except biota, are based on dry weight unless otherwise indicated. Organic analyses are not recovery corrected except for isotope dilution methods.



Your P.O. #: 303072 Your Project #: 303072

Site Location: CLIFFORD WWTP Your C.O.C. #: 932020-03-01

**Attention: Kim Wilkinson** 

**GM BluePlan Engineering Limited** 650 Woodlawn Rd W Block C, Unit 2 Guelph, ON CANADA N1K 1B8

> Report Date: 2023/11/15 Report #: R7913191

Version: 1 - Final

### **CERTIFICATE OF ANALYSIS**

# **BUREAU VERITAS JOB #: C3Y9188**

Received: 2023/11/07, 15:53

Results relate to samples tested. When sampling is not conducted by Bureau Veritas, results relate to the supplied samples tested. This Certificate shall not be reproduced except in full, without the written approval of the laboratory.

Reference Method suffix "m" indicates test methods incorporate validated modifications from specific reference methods to improve performance.

\* RPDs calculated using raw data. The rounding of final results may result in the apparent difference.

(1) Values for calculated parameters may not appear to add up due to rounding of raw data and significant figures.

### **Encryption Key**

Please direct all questions regarding this Certificate of Analysis to: Ashton Gibson, Project Manager Email: Ashton.Gibson@bureauveritas.com Phone# (905)817-5765

This report has been generated and distributed using a secure automated process.

Bureau Veritas has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per ISO/IEC 17025, signing the reports. For Service Group specific validation, please refer to the Validation Signatures page if included, otherwise available by request. For Department specific Analyst/Supervisor validation names, please refer to the Test Summary section if included, otherwise available by request. This report is authorized by Rodney Major, General Manager responsible for Ontario Environmental laboratory operations.



GM BluePlan Engineering Limited

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

#### **RESULTS OF ANALYSES OF WATER**

Bureau Veritas ID		XNN323		XNN324			XNN324		
Sampling Date		2023/11/07 12:00		2023/11/07 11:30			2023/11/07 11:30		
COC Number		932020-03-01		932020-03-01			932020-03-01		
	UNITS	LAGOON EFFLUENT	QC Batch	FILTER EFFLUENT	RDL	QC Batch	FILTER EFFLUENT Lab-Dup	RDL	QC Batch
Inorganics									
Total Ammonia-N	mg/L	6.1	9040409	3.1	0.050	9040409			
Total BOD	mg/L	6	9038584	5	2	9038584			
Total Carbonaceous BOD	mg/L	5	9038589	<2	2	9038589			
Total Kjeldahl Nitrogen (TKN)	mg/L	7.4	9041098	4.1	0.50	9039685	3.9	0.50	9039685
рН	рН	7.96	9037835	7.84		9037835			
Dissolved Phosphorus	mg/L	0.11	9046788	0.18	0.020	9046788			
Total Phosphorus	mg/L	0.20	9041080	0.27	0.020	9036869			
Total Suspended Solids	mg/L	<10	9041919	<10	10	9044252			
Alkalinity (Total as CaCO3)	mg/L	180	9037831	190	1.0	9037831			
Nitrite (N)	mg/L	0.123	9036940	0.355	0.010	9036940	0.341	0.010	9036940
Nitrate (N)	mg/L	0.31	9036940	1.09	0.10	9036940	1.08	0.10	9036940
Nitrate + Nitrite (N)	mg/L	0.44	9036940	1.45	0.10	9036940	1.42	0.10	9036940

RDL = Reportable Detection Limit

QC Batch = Quality Control Batch

Lab-Dup = Laboratory Initiated Duplicate



GM BluePlan Engineering Limited

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

# **MICROBIOLOGY (WATER)**

Bureau Veritas ID		XNN323	XNN324		
Sampling Date		2023/11/07 12:00	2023/11/07 11:30		
COC Number		932020-03-01	932020-03-01		
	UNITS	LAGOON EFFLUENT	FILTER EFFLUENT	RDL	QC Batch
Microbiological					
Escherichia coli	CFU/100mL	20	<10	10	9035464



Report Date: 2023/11/15

GM BluePlan Engineering Limited

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

#### **TEST SUMMARY**

**Bureau Veritas ID:** XNN323

Sample ID: LAGOON EFFLUENT

Matrix: Water

Collected:

2023/11/07

Shipped:

**Received:** 2023/11/07

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Alkalinity	AT	9037831	N/A	2023/11/09	Nachiketa Gohil
Biochemical Oxygen Demand (BOD)	DO	9038584	2023/11/09	2023/11/14	Gurjot Kaur
Carbonaceous BOD	DO	9038589	2023/11/09	2023/11/14	Gurjot Kaur
E.coli, (CFU/100mL)	PL	9035464	N/A	2023/11/07	Sonja Elavinamannil
Total Ammonia-N	LACH/NH4	9040409	N/A	2023/11/10	Prabhjot Kaur
Nitrate & Nitrite as Nitrogen in Water	LACH	9036940	N/A	2023/11/15	Viorica Rotaru
рН	AT	9037835	2023/11/08	2023/11/09	Nachiketa Gohil
Total Kjeldahl Nitrogen in Water	SKAL	9041098	2023/11/09	2023/11/10	Rajni Tyagi
Total Phosphorus (Colourimetric)	SKAL/P	9041080	2023/11/09	2023/11/10	Muskan
Dissolved Phosphorus	SKAL/P	9046788	2023/11/14	2023/11/15	Muskan
Total Suspended Solids	BAL	9041919	2023/11/10	2023/11/11	Razieh Tabesh

Bureau Veritas ID: XNN324

Sample ID: FILTER EFFLUENT

Matrix: Water

Collected: Shipped:

2023/11/07

**Received:** 2023/11/07

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Alkalinity	AT	9037831	N/A	2023/11/09	Nachiketa Gohil
Biochemical Oxygen Demand (BOD)	DO	9038584	2023/11/09	2023/11/14	Gurjot Kaur
Carbonaceous BOD	DO	9038589	2023/11/09	2023/11/14	Gurjot Kaur
E.coli, (CFU/100mL)	PL	9035464	N/A	2023/11/07	Sonja Elavinamannil
Total Ammonia-N	LACH/NH4	9040409	N/A	2023/11/10	Prabhjot Kaur
Nitrate & Nitrite as Nitrogen in Water	LACH	9036940	N/A	2023/11/15	Viorica Rotaru
рН	AT	9037835	2023/11/08	2023/11/09	Nachiketa Gohil
Total Kjeldahl Nitrogen in Water	SKAL	9039685	2023/11/09	2023/11/10	Rajni Tyagi
Total Phosphorus (Colourimetric)	SKAL/P	9036869	2023/11/08	2023/11/09	Muskan
Dissolved Phosphorus	SKAL/P	9046788	2023/11/14	2023/11/15	Muskan
Total Suspended Solids	BAL	9044252	2023/11/11	2023/11/13	Razieh Tabesh

Bureau Veritas ID: XNN324 Dup

Sample ID: FILTER EFFLUENT

Matrix: Water

**Collected:** 2023/11/07 Shipped:

**Received:** 2023/11/07

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Nitrate & Nitrite as Nitrogen in Water	LACH	9036940	N/A	2023/11/15	Viorica Rotaru
Total Kjeldahl Nitrogen in Water	SKAL	9039685	2023/11/09	2023/11/10	Rajni Tyagi



GM BluePlan Engineering Limited

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

### **GENERAL COMMENTS**

Each temperature is the average of up to three cooler temperatures taken at receipt

Package 1 7.3°C

Results relate only to the items tested.



#### **QUALITY ASSURANCE REPORT**

**GM BluePlan Engineering Limited** 

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

			Matrix Spike		SPIKED	SPIKED BLANK		Blank	RPD		QC Standard	
QC Batch	Parameter	Date	% Recovery	QC Limits	% Recovery	QC Limits	Value	UNITS	Value (%)	QC Limits	% Recovery	QC Limits
9036869	Total Phosphorus	2023/11/09	101	80 - 120	104	80 - 120	<0.020	mg/L	1.5	20	104	80 - 120
9036940	Nitrate (N)	2023/11/15	93	80 - 120	99	80 - 120	<0.10	mg/L	1.5	20		
9036940	Nitrite (N)	2023/11/15	98	80 - 120	108	80 - 120	<0.010	mg/L	4.1	20		
9037831	Alkalinity (Total as CaCO3)	2023/11/09			96	85 - 115	<1.0	mg/L	1.7	20		
9037835	рН	2023/11/09			102	98 - 103			0.80	N/A		
9038584	Total BOD	2023/11/14					<2	mg/L	NC	30	93	80 - 120
9038589	Total Carbonaceous BOD	2023/11/14					<2	mg/L	4.5	30	93	85 - 115
9039685	Total Kjeldahl Nitrogen (TKN)	2023/11/10	NC	80 - 120	99	80 - 120	<0.10	mg/L	5.2	20	111	80 - 120
9040409	Total Ammonia-N	2023/11/10	104	75 - 125	104	80 - 120	<0.050	mg/L	NC	20		
9041080	Total Phosphorus	2023/11/10	109	80 - 120	104	80 - 120	<0.020	mg/L	3.4	20	107	80 - 120
9041098	Total Kjeldahl Nitrogen (TKN)	2023/11/13	NC	80 - 120	101	80 - 120	<0.10	mg/L	0.17	20	96	80 - 120
9041919	Total Suspended Solids	2023/11/11			96	85 - 115	<10	mg/L	NC	20		
9044252	Total Suspended Solids	2023/11/13			97	85 - 115	<10	mg/L	NC	20		
9046788	Dissolved Phosphorus	2023/11/15	94	80 - 120	99	80 - 120	<0.020	mg/L	5.4	20	99	80 - 120

N/A = Not Applicable

Duplicate: Paired analysis of a separate portion of the same sample. Used to evaluate the variance in the measurement.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate sample matrix interference.

QC Standard: A sample of known concentration prepared by an external agency under stringent conditions. Used as an independent check of method accuracy.

Spiked Blank: A blank matrix sample to which a known amount of the analyte, usually from a second source, has been added. Used to evaluate method accuracy.

Method Blank: A blank matrix containing all reagents used in the analytical procedure. Used to identify laboratory contamination.

NC (Matrix Spike): The recovery in the matrix spike was not calculated. The relative difference between the concentration in the parent sample and the spike amount was too small to permit a reliable recovery calculation (matrix spike concentration was less than the native sample concentration)

NC (Duplicate RPD): The duplicate RPD was not calculated. The concentration in the sample and/or duplicate was too low to permit a reliable RPD calculation (absolute difference <= 2x RDL).



GM BluePlan Engineering Limited

Client Project #: 303072

Site Location: CLIFFORD WWTP

Your P.O. #: 303072 Sampler Initials: GP

### **VALIDATION SIGNATURE PAGE**

The analytical data and all QC contained in this report were reviewed and validated by:

Anastassia Hamanov, Scientific Specialist

Sonja Elavinamannil, Master of Biochemistry, Team Lead

Bureau Veritas has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per ISO/IEC 17025, signing the reports. For Service Group specific validation, please refer to the Validation Signatures page if included, otherwise available by request. For Department specific Analyst/Supervisor validation names, please refer to the Test Summary section if included, otherwise available by request. This report is authorized by Rodney Major, General Manager responsible for Ontario Environmental laboratory operations.

81	HEAU RITAS	Bureau Veritas 6740 Campobello I	Road, Mississauga, O	ntario Canada L	5N 2L8 Tel (905) 8	317-5700 Toil-free 8	100-563-6266 Fax	:(905) 817-	5777 www	bvna.com							CHAIR	N OF CUST	ODY RECORD	Page of
	otte montage (in	INVOICE TO:				RE	PORT TO:						PROJEC	T INFOR	MATION:				Laboratory Use	Only:
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Tel:	(519) 824-81	50 Fax:	(519) 824-808	9 Tet	(91	9) 824-8150 Ex	d: <b>353</b> Fax	9		L LOST	Site #:	all reg.		-	0			000		Ashton Gibson
Email:	info@gmblue		CONTRACTOR OF THE PROPERTY OF	Ema	-	ma eleciuk@g	mbluepieg ge	COTY	nut@8	phluepla	Sampled		Gr	ant	Park	mson			C#932020-03-01	
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	Regulation 153 (2011)		Other Regulation	ins	Spec	ial Instructions	circle):	2	1				soluble						tandard) TAT:	
Table			Sanitary Sew		-		- Se C	BOD			Ammang	00	n/c		~			Art and the second	d if Rush TAT is not specified): = 5-7 Working days for most tests.	X
Table	e 2 Ind/Comm C e 3 Agri/Other F			Bylaw			plea g/C	J			8	2	78		4	- ~		Please note: S	Standard TAT for certain tests such as it your Project Manager for details.	BOD and Dioxins/Furans are > 5
Table	i	PWQO	Municipality Reg 406 Tal	ble			ber (	-	S		2	_	40	+	Kalin	20		_	Rush TAT (if applies to entire sub	nission)
		Other					d Filtered (please of Metals / Hg / Cr VI	CA	S	X	(a)	7	20	. 4	3			Date Required	tT	me Required:
		iteria on Certificate of	and the second s				Field Filtered (please	BOR	-	1	Total	NO	Total		~	W		Rush Confirm	-	call lab for #)
-	Sample Barcode Label	Sample (Location	on) Identification	Date Sampl	ed Time Samp	eled Matrix		Med	-	<del></del>	٠,		1-					# of Bottles	Comm	ents
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	Night	o allusen	23/1	1/07 /	124	men o	(AMSIN)	DANK	30.0	223/2		2003	1507	15	-53	Time S	ensitive	Temperatu	O/ O Present	eal Yes No
·UNLESS	OTHERWISE AGREED TO I	N WRITING, WORK SUBMIT	TED ON THIS CHAIN	OF CUSTODY IS			ANDARD TERMS	AND COND	ITIONS. S	IGNING OF	TAIS CHAI	OF CUSTO		ENTIS			سوال	131	DI C maci	Bureau Veritas Yellow: Client
ACKNOW	LEDGMENT AND ACCEPTAL E RESPONSIBILITY OF THE	NCE OF OUR TERMS WHIC	H ARE AVAILABLE FO	R VIEWING AT V	WW.BVNA.COM/E	NVIRONMENTAL-LA	ABORATORIES/RE	SOURCES/	COC-TER	MS-AND-CC					SAMPLE	S MUST BE	KEPT CO	OL ( < 10°C) F	DOM TIME OF SAMBLING	1 1.
SAMPLE	E CONTAINED DRESERVAT	ION HOLD TIME AND BAC	VACE INFORMATION	CAN DE LOEME		2011511100111111										UNT	L DELIVE	RY TO BUREAU	J VERITAS	10/5

Bureau Veritas Canada (2019) Inc.

# 627882

APPENDIX C: LAB RESULTS – FILTER MEDIA SAMPLING



January 15, 2024

Reference No. G4795-23-11

GM Blueplan Engineering Ltd. 650 Woodlawn Road West Block C, Unit 2 Guelph, Ontario N1K 1B8

Attention: Mr. Grant Parkinson

RE: Materials Testing & Inspection

Clifford WWTP Sand Filters GMBP Project No. 323072

\_\_\_\_\_

Dear Sir,

Please find enclosed the reports for the recent materials testing and inspection services carried out for the above project.

If you have any questions or concerns regarding this report, please do not hesitate to contact this office.

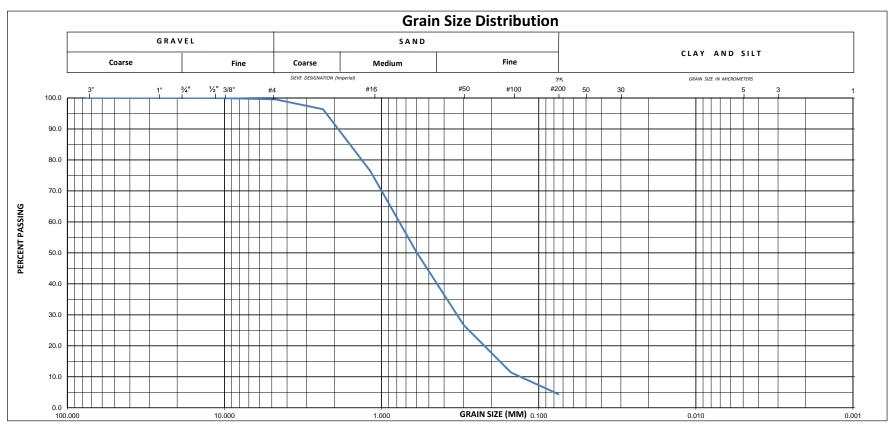
Yours very truly, JLP SERVICES INC.

Alexander Lee, M.Sc. (Eng.), P. Eng. Senior Geotechnical Engineer

AL:ak

Encls.



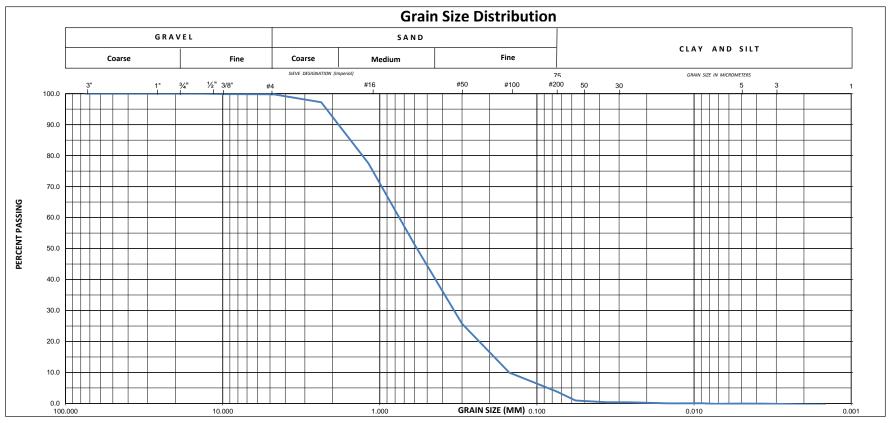


# **Classification of Sample and Group Symbol:**

SAND, trace silt, trace clay, trace gravel (SP)

Project No:	G4795-23-11	D10 =	0.14		PLASTIC PROPERTIES		
Location:	-	D30 =	0.34		Liquid Limit	% =	NP
<b>Borehole No:</b>	0	D60 =	0.82		Plastic Limit	% =	NP
Sample No:	1	Coefficien	t of Uniformity:	6.04	Plasticity Index	% =	NP
Depth (m):	0	Coefficien	t of Curvature:	3.11	Moisture Content	% =	19
Elevation (m):	0.0	Percolatio	on Time (mins/cm):	2 - 8	<b>Enclosure Number:</b>		1

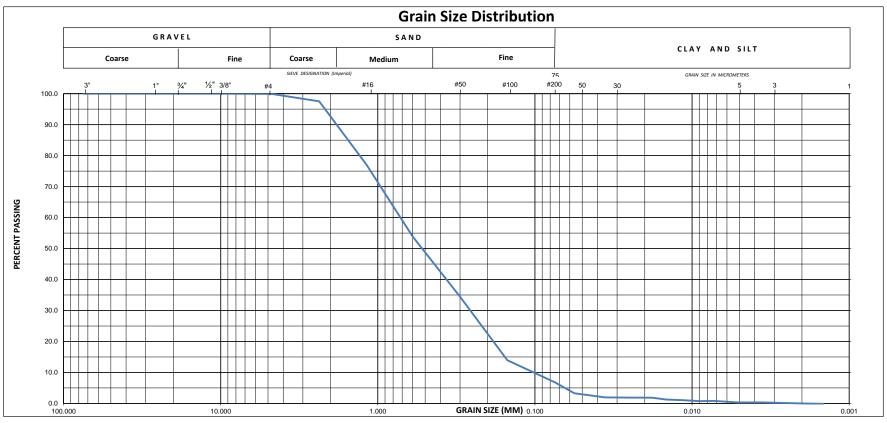




# Classification of Sample and Group Symbol: SAND, trace silt, trace gravel (SW)

Project No:	G4795-23-11	<b>D10 =</b> 0.15		PLASTIC PROPERTIES		
Location:	-	<b>D30 =</b> 0.35		Liquid Limit	% =	NP
<b>Borehole No:</b>	0	<b>D60 =</b> 0.79		Plastic Limit	% =	NP
Sample No:	2	Coefficient of Uniformity:	5.28	Plasticity Index	% =	NP
Depth (m):	0	Coefficient of Curvature:	2.93	<b>Moisture Content</b>	% =	9
Elevation (m):	0.0	Percolation Time (mins/cm):	2 - 12	<b>Enclosure Number:</b>		1

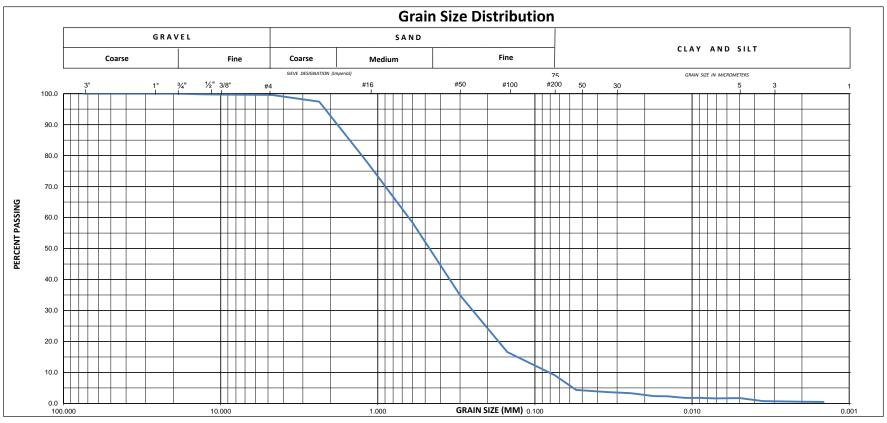




# Classification of Sample and Group Symbol: SAND, trace silt, trace gravel (SP)

Project No:	G4795-23-11	D10 =	0.11		PLASTIC PROPERTIES		
Location:	-	D30 =	0.27		Liquid Limit	% =	NP
<b>Borehole No:</b>	0	D60 =	0.75		Plastic Limit	% =	NP
Sample No:	3	Coefficient	t of Uniformity:	6.96	Plasticity Index	% =	NP
Depth (m):	0	Coefficient	t of Curvature:	3.28	Moisture Content	% =	13
Elevation (m):	0.0	Percolatio	n Time (mins/cm):	2 - 8	<b>Enclosure Number:</b>		1





# Classification of Sample and Group Symbol: SAND, trace silt, trace clay, trace gravel (SP)

Project No:	G4795-23-11	D10 =	0.08		PLASTIC PROPERTIES		
Location:	-	D30 =	0.26		Liquid Limit	% =	NP
<b>Borehole No:</b>	0	D60 =	0.65		Plastic Limit	% =	NP
Sample No:	4	Coefficient of	of Uniformity:	7.75	Plasticity Index	% =	NP
Depth (m):	0	Coefficient of	of Curvature:	4.77	Moisture Content	% =	24
Elevation (m):	0.0	Percolation	Time (mins/cm):	2 - 8	Enclosure Number:		1





# Classification of Sample and Group Symbol: SAND, trace silt, trace gravel, trace clay (SW)

Project No:	G4795-23-11	D10 =	0.16		PLASTIC PROPERTIES		
Location:	-	D30 =	0.37		Liquid Limit	% =	NP
<b>Borehole No:</b>	0	D60 =	0.84		Plastic Limit	% =	NP
Sample No:	5	Coefficien	t of Uniformity:	5.08	Plasticity Index	% =	NP
Depth (m):	0	Coefficien	t of Curvature:	2.69	Moisture Content	% =	17
Elevation (m):	0.0	Percolatio	n Time (mins/cm):	2 - 12	<b>Enclosure Number:</b>		1

APPENDIX D: COST ESTIMATES – DETAILED BREAKDOWN

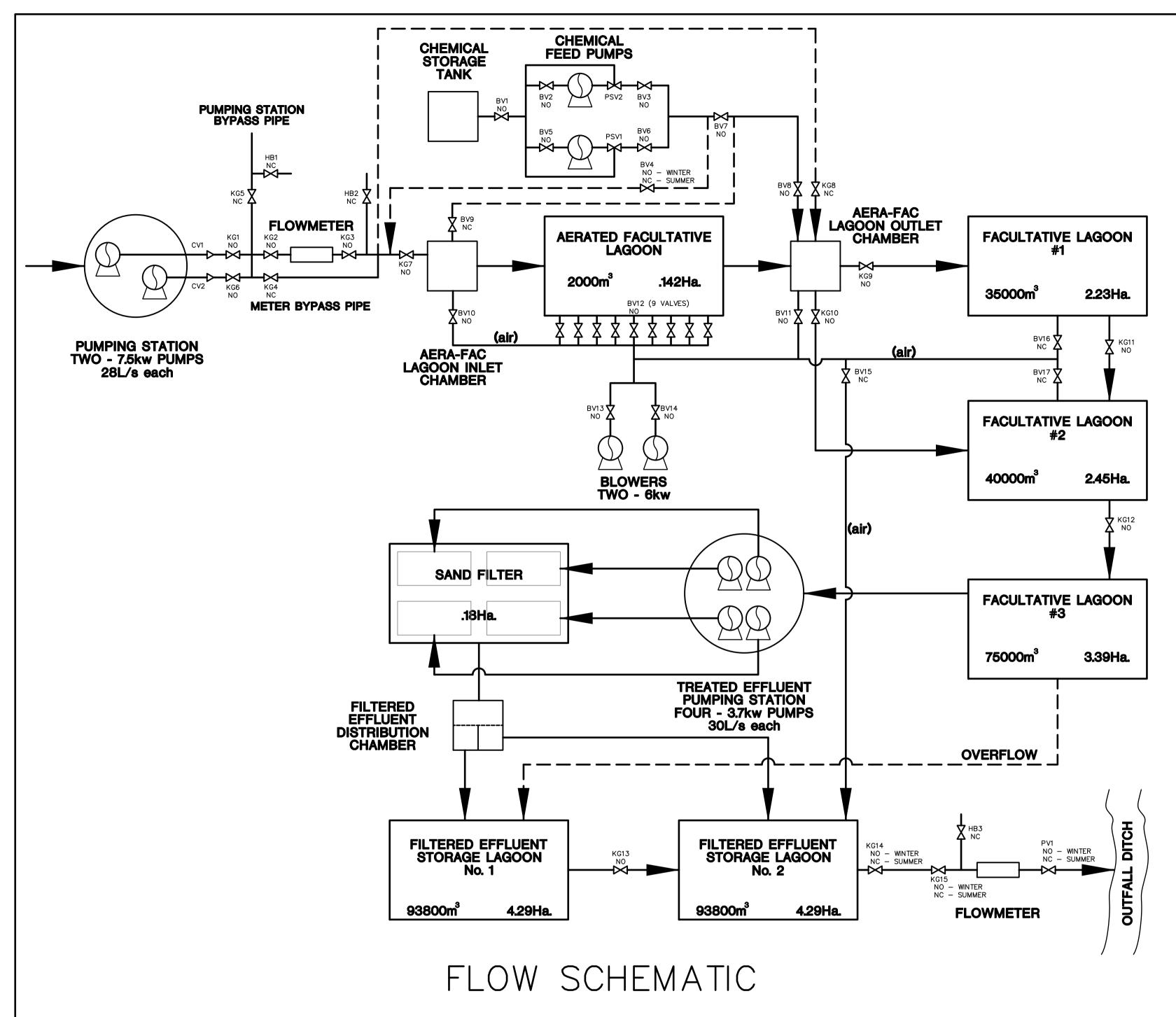
# TOWN OF MINTO CLIFFORD WWTP UPGRADES Option #1 - Partial Filter Sand Replacement

Item No.	Description of Item	Quantity	Unit	Unit Cost		Total Cost
	l on 1 - General Requirements	1		ı cosı		5031
1	General Requirements	100%	L.S.	\$ 2.000.00	\$	2,000
2	Mobilization and Demobilization	100%	L.S.	3% of Total Cost		1,760
3	Bonding and Insurance	100%	L.S.	2% of Total Cost		1,173
4	Allowance to Maintain Operation of Existing Plant during Construction	100%	L.S.	\$ -	\$	1,170
5	Legal Survey of Site to Verify Property Boundaries	0	L.S.	\$ -	\$	
		-		ements Sub-Total	\$	4,933
Divisio	on 2 - Site Construction				Ψ	4,000
6	Removals - Irrigiation Piping, Fittings, Supports. Stockpile on site.	100%	L.S.	\$ 5,000.00	\$	5,000
7	Excavate, Remove, and Dispose Off Site Top Layer of Filter Sand	0	cu.m.	\$ 14.00	\$	3,000
8	Supply and Place New Top Layer of Filter Sand	1,104	tonne	\$ 35.00		38,640
9	Spread New Top Layer of Filter Sand	1,840		\$ 35.00	_	2,760
	Re-Install Irrigation Piping and Fittings	1,840	sq.m. I.m.	\$ 5.00		9,200
11	Re-Install Irrigation Piping Supports and Fasteners	613	each	\$ 5.00		3,067
- 11				ruction Sub-Total		
Divisio		ISION 2 - S	ite Consti	uction Sub-Total	Þ	58,667
DIVISIO	on 3 - Concrete	_		I 🛕		
	not applicable	0	L.S.	\$ -	\$	
<u> </u>		DIVIS	ion 3 - Co	ncrete Sub-Total	\$	
DIVISIO	on 4 - Masonry			1.4		
	not applicable	0	L.S.	\$ -	\$	
		Divis	ion 4 - Ma	asonry Sub-Total	\$	
Divisio	on 5 - Metals					
	not applicable	0	L.S.	\$ -	\$	
		Div	vision 5 -	Metals Sub-Total	\$	
Divisio	on 6 - Wood and Plastics					
	not applicable	0	L.S.	\$ -	\$	
		sion 6 - Wo	od and P	lastics Sub-Total	\$	
Divisio	on 7 - Thermal and Moisture Protection					
	not applicable	0	L.S.	\$ -	\$	
	Division 7 - Therma	al and Moi	sture Pro	tection Sub-Total	\$	
Divisio	on 8 - Doors and Windows					
	not applicable	0	L.S.		\$	
	Divisi	on 8 - Doo	rs and Wi	ndows Sub-Total	\$	_
Divisio	on 9 - Finishes					
	not applicable	0	L.S.	\$ -	\$	
		Divis	sion 9 - Fi	nishes Sub-Total	\$	
Divisio	on 10 - Specialties				· ·	
	not applicable	0	L.S.	l \$ -	\$	
	ποι αρρποαιστο			cialties Sub-Total	\$	
Divisio	on 11 - Equipment	211101011	оро		Ψ	
Diviole	THE Equipment	Division	11 - Faui	ipment Sub-Total	\$	
Divisio	on 12 - Furnishings	Dividion	чи	ipinioni oub rotui	Ψ	-
D.111010	not applicable	0	L.S.	<b> </b> \$ -	\$	
	пос аррисаріе			l Ψ - shings Sub-Total	\$	•
Divisio	on 13 - Special Construction	DIVISION	12 - 1 ullil	Sillings Sub-Total	Þ	•
DIVISIO				I a		
	not applicable	0	L.S.	\$ -	\$	-
<u> </u>		1 13 - Spec	iai Consti	ruction Sub-Total	\$	
DIVISIO	on 14 - Conveying Systems					
	not applicable	0	L.S.	\$ -	\$	
		n 14 - Con	veying Sy	stems Sub-Total	\$	
	on 15 - Mechanical					
	not applicable	0	L.S.	\$ -	\$	
		Division	15 - Mech	nanical Sub-Total	\$	
Divisio	on 16 - Electrical					
	not applicable	0	L.S.	\$ -	\$	
		Divisio	n 16 - Ele	ectrical Sub-Total	\$	
			Consti	ruction Sub-Total	\$	63,600
Engine	eering Services					
	Detailed design, approvals, tendering, contract administration, site					
15	inspection, materials testing, testing and commissioning (10% of	100%	L.S.	10%	\$	6,360
-	construction sub-total)		-		<u> </u>	2,200
	,	Engin	eering Se	rvices Sub-Total	\$	6,360
				Sub-Total	\$	69,960
		tingency A	llowance		\$	13,992
	Con					
				excl. HST (\$2024)		83,952

# TOWN OF MINTO CLIFFORD WWTP UPGRADES Option #2 - Full Filter Sand Replacement

Item No.	Description of Item	Quantity	Unit	Unit Cost		Total Cost
	on 1 - General Requirements			0031	<u> </u>	0031
1	General Requirements	100%	L.S.	\$ 5,000.00	\$	5,000
2	Mobilization and Demobilization	100%	L.S.	5% of Total Cost	_	6,537.28
3	Bonding and Insurance	100%	L.S.	2% of Total Cost		2,614.91
4	Allowance to Maintain Operation of Existing Plant during Construction	100%	L.S.	\$ 10,000.00	\$	10,000
				ments Sub-Total	\$	24,152
Divisio	on 2 - Site Construction		•		<u> </u>	, -
6	Removals - Irrigiation Piping, Fittings, Supports. Stockpile on site.	100%	L.S.	\$ 5,000.00	\$	5,000
7	Excavate, Remove, and Dispose Entire Layer of Filter Sand	1,398	cu.m.	\$ 14.00	\$	19,578
8	Supply and Place New Full Layer of Filter Sand	2,797	tonne	\$ 35.00	\$	97,888
9	Spread New Layer of Filter Sand	1,840	sq.m.	\$ 4.50	\$	8,280
10	Re-Install Irrigation Piping and Fittings	1,840	l.m.	\$ 5.00	\$	9,200
11	Re-Install Irrigation Piping Supports and Fasteners	613	each	\$ 5.00	\$	3,067
	Divis	sion 2 - Sit	e Constru	iction Sub-Total	\$	130,746
Divisio	on 3 - Concrete					
	not applicable	0	L.S.	\$ -	\$	
		Division	on 3 - Cor	ncrete Sub-Total	\$	
Divisio	on 4 - Masonry					
	not applicable	0	L.S.	\$ -	\$	
		Divisi	ion 4 - Ma	sonry Sub-Total	\$	
Divisio	on 5 - Metals					
	not applicable	0	L.S.	\$ -	\$	
		Div	ision 5 - N	Metals Sub-Total	\$	
Divisio	on 6 - Wood and Plastics					
	not applicable	0	L.S.	0	\$	
	Divisi	ion 6 - Wo	od and Pl	astics Sub-Total	\$	
Divisio	on 7 - Thermal and Moisture Protection					
	not applicable	0	L.S.	\$ -	\$	
	Division 7 - Therma	and Mois	ture Prote	ection Sub-Total	\$	
Divisio	on 8 - Doors and Windows					
	not applicable	0	L.S.	\$ -	\$	
	Divisio	n 8 - Doors	s and Win	dows Sub-Total	\$	
Divisio	on 9 - Finishes					
	not applicable	0	L.S.	\$ -	\$	
		Divisi	ion 9 - Fin	ishes Sub-Total	\$	
Divisio	on 10 - Specialties					
	not applicable	0	L.S.	\$ -	\$	
		Division	10 - Spec	ialties Sub-Total	\$	
Divisio	on 11 - Equipment					
		Division	11 - Equip	oment Sub-Total	\$	
Divisio	on 12 - Furnishings					
	not applicable	0	L.S.	\$ -	\$	
		Division 1	2 - Furnis	hings Sub-Total	\$	
Divisio	on 13 - Special Construction					
	not applicable	0	L.S.	\$ -	\$	
		13 - Specia	al Constru	iction Sub-Total	\$	
Divisio	on 14 - Conveying Systems					
	not applicable	0	L.S.	\$ -	\$	
		14 - Conv	eying Sy	stems Sub-Total	\$	
Divisio	on 15 - Mechanical					
	not applicable	0	L.S.	\$ -	\$	
		Division '	15 - Mech	anical Sub-Total	\$	
Divisio	on 16 - Electrical					
	not applicable	0	L.S.	\$ -	\$	
		Divisio	n 16 - Elec	ctrical Sub-Total	\$	
			Constru	uction Sub-Total	\$	154,898
Engin	eering Services					
	Detailed design, approvals, tendering, contract administration, site					
15	inspection, materials testing, testing and commissioning (10% of	100%	L.S.	10%	\$	15,490
	construction sub-total)	<u> </u>			L	
		Engine	erina Se	rvices Sub-Total	\$	15,490
			<u> </u>			
				Sub-Total	\$	170,388
	Cont	ingency A			_	170,388 34,078

APPENDIX E: SELECTED DESIGN DRAWINGS



TREATMENT EFFICIENCY							
Parameter Raw Sewage Final Effluent % Removed							
Biochemical Oxygen Demand	170 mg/l	7.5	95.6				
Suspended Solids	170 mg/l	7.5	95.6				
Ammonia	20 mg/l	2	90.0				
Phosphorus	8 mg/l	.7	91.2				

EFFLUENT OBJECTIVES AND

	Allowable Discharge m <sup>3</sup> /d	Proposed Maximum Daily Discharge rate in m <sup>3</sup> /d	Total Volume Discharged in m <sup>3</sup>	
NOVEMBER	470	470	14,100	
DECEMBER	734	734	22,754	
JANUARY	1,028	1,028	31,868	
FEBRUARY	1,075	1,075	30,100	
MARCH	1,970	1,970	61,070	
APRIL	1,345	1,345	22,608	* See
TOTAL			182,500	

DISCHARGE RATE TO RECEIVING STREAM

DESIGN POPULATION	
DESIGN	1100
1993	720

ORGANIC	LOADING	
DESIGN	(kg/day)	85
1993	(kg/day)	56

SEWAGE PUMPING STATION	
No. OF PUMPS	2
KILOWATTS	7.5
CAPACITY (I/s) each	28

AERATED FACULTATIVE LAGOON	
- SURFACE AREA	0.142 ha.
- LIQUID DEPTH	3.0 m
- CAPACITY	2500 m <sup>3</sup>
- HYDRAULIC RETENTION TIME (HRT)	5 days

BLOWERS	
No. OF BLOWERS	2
KILOWATTS	6
AIRFLOW (I/s) each	99.0

FACULTATIVE LAGOONS	No. 1	No. 2	No. 3	TOTAL
- SURFACE AREA, ha	2.23	2.45	3.39	8.07
- LIQUID DEPTH, m	1.80	1.80	2.40	
- CAPACITY, m <sup>3</sup>	35000	40000	75000	150000
- HYDRAULIC RETENTION TIME (HRT), days	70	80	150	300

FILTER MEDIA			
- 760 mm SAND	0.20 mm EFFECTIVE SIZE		
	3.0 UNIFORMITY COEFFICIENT		
– 75 mm	5 mm CRUSHED STONE		
– 75 mm	10 mm CRUSHED STONE		
– 75 mm	15 mm CRUSHED STONE		
– 75 mm	20 mm CRUSHED STONE		

SAND FILTER		
- NUMBER OF CELLS		4
- AREA PER CELL	(ha)	0.046
- TOTAL AREA	(ha)	0.18
- HYDRAULIC LOADING RATE	(m³/ha/day)	5600
- APPLICATION RATE (MAX)	(m³/day)	1000

FILTERED EFFLUENT STORAGE LAGOONS	No. 1	No. 2	TOTAL
- SURFACE AREA, ha	4.29	4.29	8.58
- LIQUID DEPTH, m	2.40	2.40	
- CAPACITY, m <sup>3</sup>	93800	93800	187600
- HYDRAULIC RETENTION TIME (HRT), days	187.6	187.6	375.2

TREATED EFFLUENT PUMPING STATION	
No. OF PUMPS	4
KILOWATTS	3.7
CAPACITY (I/s) each	30

MUNICIPALITY KEY PLAN

NOT TO SCALE

1. MAX MONTHLY DISCHARGE FOR APRIL IS 40,350 m<sup>3</sup>

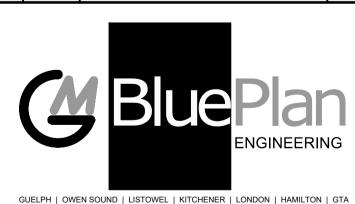
BENCH MARKS :

NOTES:

THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND, WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED.

BEFORE STARTING WORK, THE CONTRACTOR SHALL INFORM HIMSELF OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR ANY DAMAGE TO THEM.

NO. DATE REVISION DESCRIPTION CH'K



650 WOODLAWN ROAD WEST, BLOCK C, UNIT 2, GUELPH, ON N1K 1B8 TEL. 519-824-8150 www.gmblueplan.ca

TOWN OF MINTO

WASTEWATER TREATMENT PLANT

CLIFFORD

FLOW SCHEMATIC & DESIGN DATA

